

## PRELIMINARY GEOTECHNICAL REPORT

# HENNEPIN COUNTY MEDICAL EXAMINERS SITE

BLOOMINGTON, MN

**DECEMBER 19, 2016** 

Prepared for: Hennepin County Public Works 701 Fourth Avenue South, Suite700 Minneapolis, MN 55415

WSB PROJECT NO. 03392-010



### PRELIMINARY GEOTECHNICAL REPORT

# PROPOSED HENNEPIN COUNTY MEDICAL EXAMINERS BUILDING 6701 WEST 78<sup>TH</sup> STREET BLOOMINGTON, MINNESOTA

## FOR HENNEPIN COUNTY

**December 19, 2016** 

### Prepared by:

WSB & Associates, Inc. 540 Gateway Boulevard Burnsville, MN 55337 (952)-737-4660

### **CERTIFICATION**

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the State of Minnesota.

Darin E. Hyatt, PE

Date: December 19, 2016 Lic. No. 41316





540 Gateway Boulevard | Suite 100 | Burnsville, MN 55337 | (952) 737-4660

December 19, 2016

Ms. Brianna D. Boos Hennepin County Public Works Environment and Energy Department Land and Water, Contaminated Lands Unit 701 Fourth Avenue South, Suite 700 Minneapolis, MN 55415-1842

Re: Preliminary Geotechnical Report

Hennepin County Medical Examiners Site

6701 West 78<sup>th</sup> Street Bloomington, MN

WSB Project No. 03392-010

Dear Ms. Boos:

We have conducted a preliminary geotechnical subsurface exploration program for the above referenced project. This report contains our soil boring logs, an evaluation of the conditions encountered in the borings and our preliminary recommendations for suitable foundation type, a range of allowable soil bearing pressures for footing design, and other geotechnical related design and construction considerations.

If you have any questions concerning this report or our preliminary recommendations please call us at (952) 737-4660.

Sincerely,

WSB & Associates, Inc.

Darin Hyatt, PE Senior Geotechnical Engineer

Attachment

Preliminary Geotechnical Report

DEH/tmw

Joe Carlson, EIT Graduate Geotechnical Engineer

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#### 1. INTRODUCTION

### 1.1 Project Location

This site is located at 6701 West 78<sup>th</sup> Street in Bloomington, Minnesota. The boring locations can be seen on the map in *Appendix A*.

The area was a vacant lot at the time of drilling with sporadic shrubs throughout. Nine Mile Creek runs along the western end of the site and there is a pond located at the south end of the site. It is our understanding that a structure that previously occupied this land was demolished and removed from the site.

#### 1.2 Project Description

Very little design information is available at this time, however, we understand the building at this site will generally be less than three (3) stories, slab-on-grade structure with a finished floor elevation within about two feet (2') of existing grades. We have assumed wall loads will be less than 12 kips per linear foot and column loads will be less than 250 kips each. It is also assumed, underground utilities will have invert elevations within fifteen feet (15') of existing grades.

WSB has developed preliminary foundation recommendations for this project. When the architect and/or structural engineer develops additional information about final design column loadings, building configuration, or other significant factors, the recommendations presented herein may no longer apply. We anticipate that additional soil borings and a final geotechnical report will be completed when the final design information is completed. We recommend the additional soil borings be performed within the planned building and pavement areas to better characterize the subsurface conditions at this site.

#### 1.3 Purpose and Project Scope of Services

Ms. Brianna Boos with the Hennepin County Public Works authorized our proposal. In order to assist the City in evaluating this site for potential development, we have completed a subsurface exploration program and prepared a preliminary geotechnical report for the referenced site. This stated purpose was a significant factor in determining the scope and level of service provided. Should the purpose of the report change the report immediately ceases to be valid and use of it without WSB's prior review and written authorization shall be at the user's sole risk.

Our authorized scope of work has been limited to:

- 1. Mobilization / Demobilization of a Truck Mounted Drill Rig.
- 2. Clearing underground utilities utilizing the Gopher State One Call.
- 3. Drilling 22 standard penetration borings to depths of about 25 feet.
- 4. Sealing the borings per Minnesota Department of Health procedures.
- 5. Perform soil classification and analysis.
- 6. Review of readily available project information and geologic data.
- 7. Providing this preliminary geotechnical report containing:
  - A. Summary of our initial findings.
  - B. Discussion of subsurface soil and groundwater conditions and how they may affect potential future construction.
  - C. Estimated range of allowable bearing capacities of the soils.
  - D. Preliminary recommendations for foundations.
  - E. A discussion of soils for use as structural fill and site fill.



#### 2. PROCEDURES

### 2.1 Boring Layout and Soil Sampling Procedures

Hennepin County Public Works requested we complete 22 soil borings at this site, recommended the boring depths and selected the desired locations. The boring locations were staked using existing site features as guides and elevated by our drilling crew. Bore hole elevations were referenced to the top of the top nut of a hydrant located on West 78<sup>th</sup> Street near the western entrance to the site. The elevation of that benchmark was indicated as 842.73 on a site plan provided to us.

We drilled the borings on November 17, 18, 21 and 22, 2016, with a truck-mounted CME-55 drill rig operated by a two-person crew. The drill crew advanced the borings using continuous hollow stem augers. Drilling methods, crew chief, depths, sampling interval, casing usage, groundwater observations, test data and other drilling information are indicated on the boring logs.

Generally, the drill crew sampled the soil in advance of the auger tip at two and one-half (2½) foot intervals of depth to fifteen feet (15') and at five foot (5') intervals thereafter. The soil samples were obtained using a split-barrel sampler which was driven into the ground during standard penetration tests in accordance with ASTM D 1586, Standard Method of Penetration Test and Split-Barrel Sampling of Soils.

The materials encountered were described on field logs and representative samples were containerized, and transported to our laboratory for further examination and testing.

The samples were visually examined to estimate the distribution of grain sizes, plasticity, consistency, moisture condition, color, presence of lenses and seams, and apparent geologic origin. We classified the soils according to type using the Unified Soil Classification System (USCS). A chart describing the Unified Soil Classification System is included in *Appendix A*.

#### 2.2 Groundwater Measurements and Borehole Abandonment

The drill crew observed the borings for free groundwater while drilling and after completion. These observations and measurements are noted on the boring logs. The crew backfilled the borings with soil cuttings; to comply with Minnesota Department of Health regulations.

### 2.3 Boring Log Procedures and Qualifications

The subsurface conditions encountered by the test borings are illustrated on the attached boring logs. Similar soils were grouped into the strata shown on the boring logs, and the appropriate estimated USCS classification symbols were also added. The depths and thickness of the subsurface strata indicated on the boring logs were estimated from the drilling results.

The transition between materials (horizontal and vertical) is approximate and is usually far more gradual than shown. Information on actual subsurface conditions exists only at the specific locations indicated and is relevant only to the time exploration was performed. Subsurface conditions and groundwater levels at other locations may differ from conditions found at the indicated locations. The nature and extent of which would not become evident until exposed by construction excavation. These stratification lines were used for our analytical purposes and, due to the aforementioned limitations, should not be used as a basis of design or construction cost estimates.



#### 3. EXPLORATION RESULTS

### 3.1 Site and Geology

At the time of drilling, the site was an undeveloped lot with sporadic shrubs.

Based on review of online topographic maps, this site appears to gently slope to the southeast. Nine Mile Creek traverses the western side of the site.

The Hennepin County Geologic Atlas indicates the surficial geology of the area is mostly organic deposits much of which have been drained and filled.

### 3.2 Subsurface Soil and Groundwater Conditions

#### Soil Borings

The boring profile generally consisted of fill overlying alluvial and glacially deposited soils.

The fills encountered ranged from about 2 to 12 feet below grade and consisted of a mixture of lean clay, silty sand and sands. Below the fill in Boring PB-15, buried topsoil was encountered to a depth of about 5 feet.

Below the fills and buried topsoil, we encountered deposits consisting of sands and silty sands, lean clays and to a lesser extent fat clays. These soils were generally brown to gray in color and ranged from moist to saturated or waterbearing.

The soils encountered were generally similar to the soils described in the Geologic Atlas.

#### **Test Pits**

Six test pits were excavated to better evaluate environmental contamination concerns. Similar to the soil borings the test pits encountered fill soils consisting of silty sand and sand to depths of about 2 to 7 ½ feet. It should be noted that the native soils below the fill in test pit 1 was dark in color. An organic test on that material indicated it had about 3 percent organic material classifying it as slightly organic. Underlying the fill naturally deposited soils consisting of silty sand, sand and silt were encountered.

### 3.3 Strength Characteristics

The penetration resistance N-values of the materials encountered were recorded during drilling and are indicated as blows per foot (BPF). Those values provide an indication of soil strength characteristics and are located on the boring log sheets. Also, visual-manual classification techniques and apparent moisture contents were also utilized to make an engineering judgment of the consistency of the materials. The following table presents a summary of the penetration resistances in the soils and remarks regarding the material strengths of the soils.

Soil Type	Classification	Penetration Resistances	Remarks
Fill	Mixed Soils	3 - 28 BPF, average 13 BPF	Variable compaction
Coarse Alluvium	SP, SM	3 to 20 BPF	Very loose to medium dense
Fine Alluvium	CL, CH	1 to 12 BPF	Very soft to firm
Till	CL	6 to 22 BPF	Soft to hard

**Table 1: Penetration Resistances** 

The preceding is a generalized description of soil conditions at this site. Variations from the generalized profile exist and should be assessed from the boring logs, the normal geologic character of the deposits, and the soils uncovered during site excavation.

#### 3.4 Groundwater Conditions

WSB took groundwater level readings in the exploratory borings, reviewed the data obtained, and discussed its interpretation of the data in the text of the report. Note that groundwater levels may fluctuate due to seasonal variations, e.g. precipitation, snowmelt and rainfall, and/or other factors not evident at the time of measurement.

Our borings were only left open for a short period of time; as such, groundwater levels may not have had sufficient time to stabilize at their hydrostatic level.

Table 2 below is a summary of the estimated water levels at our borings.

**Table 2: Groundwater Measurements** 

Boring No.	Ground Surface Elevation	Depth to Groundwater after Drilling	Estimated Groundwater Elevation
PB-1	839.4	15	824 ½
PB-2	831.6	23	809
PB-3	829.6	23	807
PB-4	826.4	NE	
PB-5	825.9	8	818
PB-6	832.2	18 ½	814
PB-7	830.8	15	816
PB-8	828.8	22	807
PB-9	826.1	25	801 ½
PB-10	825.1	9	816 ½
PB-11	830.9	18 ½	812 ½
PB-12	829.8	12	818
PB-13	828.0	8	820
PB-14	825.7	8	818
PB-15	825.4	9	816 ½
PB-16	828.1	15 ½	813
PB-17	826.8	9 ½	817 ½
PB-18	825.9	11 ½	814 ½
PB-19	824.3	22	802 ½
PB-20	825.7	18	808
PB-21	825.1	11	814 ½
PB-22	824.4	19	805 ½

Groundwater Depths and Elevations are rounded to the highest ½ foot. NE – indicates groundwater not encountered during drilling and sampling.



### PL2023-193 PL202300193

As can be seen in the table above, water levels observed during drilling varied widely from about elevation 801 ½ to 824 1/2. Piezometers would allow for a more long term monitoring of water levels. Piezometer installation was beyond the scope of this evaluation. Based on information provided on a survey provided to us, the pond on the south side of the site had a water elevation of 820.5 feet in July of 2014. That same survey indicated a delineated wetland adjacent to Nine Mile Creek had a limit of about 820 feet in July of 2014. It is our opinion that the hydrostatic water level at this site will be near that of the pond and Nine Mile Creek.



#### 4. PRELIMINARY ENGINEERING ANALYSIS AND RECOMMENDATIONS

The existing fills were mostly composed of sands with silt and silty sands and in a few instances clayey soils were noted. In some of the fill we noted pieces of wood, limestone, cobbles, and pieces of concrete or bituminous. It is likely these fills were placed following removal of organic soils. With the exception of Boring PB-15, the borings did not encounter any materials containing appreciable organic matter within or beneath the fill. Given the site was occupied by a previous structure it is possible that some of the fill, at least within previous building footprints was placed as a structural fill. However, it is unknown what occurred during and following demolition of the previous structure. Variable blows per foot (BPF) were also encountered within the fill, we are uncertain of the exact footprint of previous buildings or the magnitude of previous structural loadings, no observation or compaction testing documentation was made available to us and in one boring buried topsoil was encountered. As such, the existing fill is considered undocumented. It is our opinion, placing a building of the magnitude proposed on or within the existing fill would have a high risk of detrimental settlement.

Flexible (bituminous paved) parking lots are lightly loaded and more tolerant of movement compared to a building. Therefore consideration can be given to leaving the existing fill in place beneath proposed parking lots. However, with this approach the owner would need to accept a slight risk of increased long-term settlement.

In general, the native soils underlying the fill appear suitable for support of potential structures but they too would need to be further evaluated, especially where very soft and soft clayey soils exist.

Consideration could also be given to supporting proposed structures on deep or intermediate foundations such as driven pile or Geopiers®. These options would not require the removal of the existing fill.

It is our opinion that groundwater could be encountered by excavations at this site. Dewatering should be anticipated.

### 4.1 Preliminary Building Area Preparation

Unless information regarding the placement and compaction of the existing fill is provided to us for evaluation, we recommend the existing fill be removed from beneath the new building and an appropriate oversize area, and be replaced with compacted backfill.

Table 3 indicates the approximate minimum excavation depths to remove existing fill soils based on the findings of our soil borings. Excavation depths and bottom elevations were rounded to the lowest 1/2 foot/elevation. Those depths will likely vary and should be observed and adjusted during construction. Furthermore, it may be necessary to extend excavations to include partial removal of the soft natural clays depending on footing elevations, structural loads and condition of the clays at the time of construction.



Table 3. Approximate Minimum Excavation Depths at the Boring Locations

Boring	Ground Surface Elevation	Approximate Min. Excavation Depth*, feet	Approximate Bottom Elevation
PB-1	839.4	12	827
PB-2	831.6	7	824
PB-3	829.6	7	822 1/2
PB-4	826.8	9	817 1/2
PB-5	825.9	7	818 ½
PB-6	832.2	7	825
PB-7	830.8	9	821 ½
PB-8	828.8	10	818 ½
PB-9	826.6	11	815 ½
PB-10	825.1	7	818
PB-11	830.9	12	818 ½
PB-12	829.8	9	820 ½
PB-13	828.0	7	821
PB-14	825.7	7	818 ½
PB-15	825.4	5	821
PB-16	828.1	7 ½	820 ½
PB-17	826.8	9	817 1/2
PB-18	825.9	7	818 ½
PB-19	824.3	9	815
PB-20	825.7	7	818 ½
PB-21	825.1	7	818
PB-22	824.1	5	819

<sup>\* -</sup> Excavation depths may vary depending on the condition of the exposed soils at the time of construction and on final design grades and loads.

#### 4.2 Preliminary Foundation Recommendations

It is our opinion that the buildings may be supported on conventional spread footings bearing on naturally occurring firm clays or medium dense sands or structural fill if it is determined to have been engineered. It is our opinion the footings throughout may be designed for net allowable soil bearing pressures ranging from 2,000 to 3,500 pounds per square foot (psf), depending on building plans and site preparation.

### 4.3 Preliminary Pavement Recommendations

We recommend any organic soils be removed from within 3 feet of the top of subgrade elevation. Surface compaction of the pavement areas should then be completed. The surface compaction should be observed and tested.

### 4.3 Additional Soil Borings and Recommendations

Given the size of the site, the spacing of our borings and the lack of specific design information, we recommend additional soil borings be performed once building locations and structural loadings have been established, to further evaluate the site. When a final geotechnical report is prepared, we will provide more detailed recommendations and discuss other geotechnical related items including construction safety, field observations and testing, and plan review and remarks.



### 5. STANDARD OF CARE

The preliminary recommendations and opinions contained in this report are based on our professional judgment. The soil testing and geotechnical engineering services performed for this project have been performed with the level of skill and diligence ordinarily exercised by reputable members of the same profession under similar circumstances, at the same time and in the same or a similar locale. No warranty, either express or implied, is made.

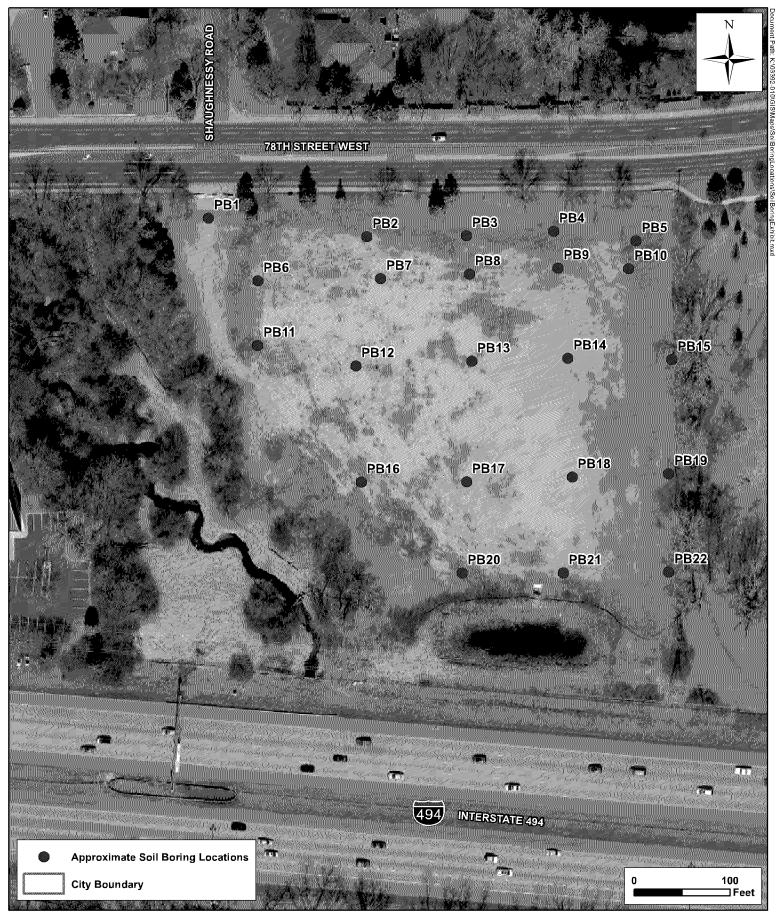


### APPENDIX A

Boring Location Map
Test Pit Map
Logs of Penetration Test Borings
Symbols and Terminology on Test Boring Log
Notice to Report Users Boring Log Information
Unified Soil Classification Sheet (USCS)



PL2023-193 PL202300193



Soil Boring Location Sketch
Preliminary Geotechnical Report
Proposed Hennepin County Medical Examiners Building
6701 W. 78th Street
Bloomington, MN







Test Pit Location
Preliminary Geotechnical Report
Proposed Hennepin County Medical Examiners Building
6701 West 78th Street
Bloomington, MN 55439





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### LOG OF TEST BORING

**BORING NUMBER PB 2** PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 831.6 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. GEOLOGIC MC DD LL PL (%) (pcf) (%) (%) **USCS** DESCRIPTION OF MATERIAL Ν M ORIGIN (ft) (ft) No. TYPE (%) FILL, mostly Silty Sand, a little Lean Clay, Fill brown, dark brown HSA 831 -830 829 13 SB 828 827 28 SB 3 826 825 SP SAND WITH GRAVEL, fine to medium Coarse Alluvium grained, brown, moist, medium dense 824 18 SB 823 822 10 16 5 SB821 11 820 12 SB13 -819 -818817 15 LEAN CLAY WITH SAND AND A LITTLE CL Glacial Till 12 7 SB GRAVEL, dark gray, moist, firm to hard, a few 16-816 lenses of water bearing sand 17-815 18 -81419--813 20-812 14 SB21--811 22--810  $\bar{\Delta}$ -809 23 --808 24-25 807 SB End of Boring 25.0 ft. 806 WATER LEVEL MEASUREMENTS START: 11/22/2016 END: 11/22/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/22/2016 11:45 am 25 24.5 23.0 808.6 3 1/4" HSA 0' - 24.5' Notes:



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BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ

### LOG OF TEST BORING

BORING NUMBER PB 5 PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 825.9 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. **GEOLOGIC** MC DD LL (%) (pcf) (%) **USCS** DESCRIPTION OF MATERIAL Ν M PL ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, mostly Sand, a little Gravel, dark brown, brown HSA 825 824 823 18 SB 822 821 14 SB 3 820 819 SP SAND WITH GRAVEL, medium to fine Coarse Alluvium grained, brown, wet to water bearing at 8', loose  $\bar{\Delta}$ 10 818 to medium dense 4 SB 817 10 -816 14 5 SB -815 11 -814 12 SB13 -813 20 6 812 811 15 SAND, fine to medium grained, brown, water SP Coarse Alluvium 16 7 SB bearing, medium dense 16 810 17 809 18 808 807 19-SAND WITH GRAVEL, medium to fine SP Coarse Alluvium grained, brown, water bearing, medium dense 20-806 16 8 SB805 21-22-804 803 23 -802 24 Glacial Till LEAN CLAY WITH SAND AND A LITTLE CL GRAVEL, dark gray, moist, firm 9 25 801 SB End of Boring 25.0 ft. 800 WATER LEVEL MEASUREMENTS START: 11/22/2016 END: 11/22/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/22/2016 2:15 pm 25 24.5 8.0 817.9 3 1/4" HSA 0' - 24.5 Notes:



		: HCPW - 6701 03392-010	W 78th Stre		ROJECT LOG URFACE EL							ORIN			PA	GE :	1 OF
DEPTH EL		DESC	RIPTION C	F MATER	IAI	USCS		OGIC	N	WL		/PLE	L, MC	ABOI DD	RATO	PL	rest I
(ft) (	ft)					USES		GIN		^	No.	TYPE	(%)	(pcf)	(%)	(%)	
1 + 8	31	FILL, a mixt Gravel			l, a little			ill ill			1	HSA					
$2 \stackrel{T}{+} 8$	30																
$3\overline{\perp}8$	29								15		2	SB					
4 + 8	28																
5—8	27								17		3	SB					
6 + 8																	
7-8		LEAN CLA GRAVEL, b	Y WITH SA rown, mois	AND AND	A LITTLE	CL	Glaci	al Till									
8 + 8									6		4	SB	11	121			
10 + 8		LEAN CLA	V WHTH C	ANID ANID	A LITTLE	CT	- CI ·	al Titt									
11 + 8		GRAVEL, d				CL	Glaci	al Till	9		5	SB					
12 - 8	20																
13 + 8	19								7		6	SB					
14 + 8	18																
15—8									9		7	SB					
16 - 8																	
17—8 18—8																	
19 - 8										Ā							
20 - 8																	
21 + 8	11								10		8	SB					
22 + 8	10																
23 + 8	09																
24—8		SAND WITI medium grai	H A LITTL ned, gray, v	E GRAVEI	L, fine to g, medium	SP	Coarse A	Alluvium									
25 - 8		dense End of Borir							14	$\Box$	9	SB					
26—8	00	***	T 101 404	3 4 GI 375	The control of the co			ar				<u> </u>		L			
			LEVEL ME			TTTAT	LED	START:		//201		w Chie			17/20 Logge		
DATE 1/17/2016	12:05 px	SAMPLED DEPTH m 25	CASING DEPTH 24.5	CAVE-IN DEPTH	WATER DEPTH 18.5	WAT ELEVA 813	ATION	METHO 3 1/4" H		_ 24	J. T	atro	1.		DAJ	ч <b>л</b> у.	
1/1 //2016	12:05 pi	<u>n 25</u>	24.5		18.5	813	<b>).</b> /	3 1/4" H	.SA 0'	- 24.	.a Not	es:					



PROJECT NA CLIENT/WSE			W 78th Stre		PROJECT LO SURFACE EL							ORIN			PA	GE :	1 OF
DEPTH ELEV.		DESCI	RIPTION O	F MATER	IAL	USCS		LOGIC	N	WL		/PLE				PL	
(ft) (ft)		FILL, mostly Lean Clay, b	Sand, a litt	le Silty San	ıd, a little			ill		1	No.	TYPE HSA	MC (%)	(pcf)	LL (%)	PL (%)	
2 - 829									1.4		2	cp.					
3 — 828 4 — 827									14		2	SB					
5—826 6—825									11		3	SB					
7—824 8—823									24		4	SB					
9 - 822 $10 - 821$		SAND WITI grained, brow	H GRAVEL vn, moist, le	, fine to me	edium lium dense	SP	Coarse A	Alluvium									
11 — 820 12 — 819									9		5	SB					
13 - 818									12		6	SB					
14 — 817 15 — 816		SAND, fine	to medium	grained, bro	own, water	SP	Coarse A	Alluvium	13	Δ	7	SB					
16—815 17—814		bearing, med	ium dense						13		,	20					
$ \begin{array}{c}                                     $		SAND WITI	I CD AVEL	Coate m	a disana	SP	Canas	Alluvium									
20 - 811 $21 - 810$		grained, brov	vn, water be	earing, loos	e	Sr	Coarse A	Anuvium	7		8	SB					
22 - 809		SANDY LE LITTLE GR			ND AND A	CL	Glaci	al Till									
23 - 808 24 - 807			, ,	. /													
25—806 - 26—805	<i>V////</i>	End of Borin	g 25.0 ft.						8		9	SB	H				
		WATER	LEVEL ME	EASUREMI	ENTS	1	L	START:	11/2	LЦ 2/201	.6		END:	11/	 22/20	L 16	<u> </u>
DATE T	ME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER DEPTH	WAT ELEVA		метно			Cre	w Chie		I	Logge DAJ		
.1/22/2016 9:4	5 am	25	24.5		15.0	815	5.8	3 1/4" H	ISA 0'	- 24.	_						



**BORING NUMBER PB 8** PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 828.8 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. GEOLOGIC MC DD LL (%) (pcf) (%) **USCS** DESCRIPTION OF MATERIAL Ν M PL ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, mostly Sand with Gravel, a little Silty Sand, brown HSA 828 827 826 16 SB 825 824 18 SB 3 823 822 821 18 4 SB 820 819 LEAN CLAY, gray, moist, firm CL Fine Alluvium 12 5 SB 818 11 12 817 SAND WITH GRAVEL, fine to medium SP Coarse Alluvium grained, brown, wet, loose 10 SB13 -816 BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ -815 -814 15 LEAN CLAY, dark gray, wet, soft CL Fine Alluvium 6 7 SB 16 813 17 812 SAND WITH GRAVEL, fine to medium SP Coarse Alluvium grained, brown, wet to water bearing at 22', 18 -811medium dense 19--81020-809 20 SB808 21  $\nabla$ 22--807 -806 23 -805 24 25 804 16 SB End of Boring 25.0 ft. 803 WATER LEVEL MEASUREMENTS START: 11/22/2016 END: 11/22/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/22/2016 8:05 am 25 24.5 22.0 806.8 3 1/4" HSA 0' - 24.5 Notes:



PROJECT NAME: HCPW - 6701 W 78th Street **BORING NUMBER PB 9** PROJECT LOCATION: Bloomington, MN CLIENT/WSB #: 03392-010 SURFACE ELEVATION: 826.1 ft PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. **GEOLOGIC** MC DD LL PL (%) (pcf) (%) (%) **USCS** DESCRIPTION OF MATERIAL N M ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, mostly Sand, Sand with Gravel, brown HSA 825 824 823 12 SB 822 821 14 3 SB 820 -819 818 12 SB817 -816 10 5 SB-815 11 LEAN CLAY WITH SAND AND A LITTLE CL Glacial Till GRAVEL, dark gray, moist to wet, firm to hard 814 12-SB13 -813 14 BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ -812 15 811 16 7 SB 16-810 17-809 18 808 807 19-20-806 9 SB21 805 22--804 23 803 -802 24-25 801 SB End of Boring 25.0 ft. 800 WATER LEVEL MEASUREMENTS START: 11/22/2016 END: 11/22/2016 Crew Chief: SAMPLED CASING CAVE-IN WATER Logged By: WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 3 1/4" HSA 0' - 24.5' Notes: 11/22/2016 8:50 am 25 24.5 25.0 801.1



- 1	T/WSB	n. <b>0</b> 5	372-010			URFACE ELI						SAN	/PLE	T.4	BOI	RATO	GE :	
DEPTH (ft)	ELEV. (ft)		DESCI	RIPTION O	F MATER	IAL	USCS	GEOL ORI	OGIC GIN	N	WL		ТҮРЕ	MC	DD	LL	PL (%)	
+	- - - 824		FILL, a mixt Gravel, brow	ure of Sand n, dark bro	, Sand with wn	Silt and		Fi	ill				HSA	(70)	(pcr)	(70)	(70)	
+	-823 - -822									27		2	SB					
4- -	- -821 -																	
Ŧ	-									28		3	SB					
ł	-		medium grai	ned, brown,	E GRAVEI , wet to wat	., fine to er bearing at	SP	Coarse A	Alluvium	12		4	SB					
+	819 818 818 SAND WITH A LITTLE GRAVEL, medium grained, brown, wet to wate 9', medium dense  816 815 814 813 810 810 809 808 808 807 806 806 807 806 808 807 806 808 807 808 808 808 807 808 808 808 808						⊻											
+	-									12		5	SB					
13-	- 812 -									16		6	SB					
ł	-									14		7	SB					
+	-																	
+	-																	
Ŧ	-		LEAN CLA GRAVEL, o	Y WITH S <i>A</i> lark gray, m	AND AND noist, hard	A LITTLE	CL	Glacia	al Till	18		8	SB					
+	-																	
24	- 801 																	
25— 26—	800  799	/////	End of Borin	g 25.0 ft.						18		9	SB					
·············			WATER	LEVEL ME	EASUREME	ENTS			START:	11/2	1/201	.6		END:	11/	21/20	16	
DATE	TI	ME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER DEPTH	WAT ELEVA	TER ATION	МЕТНО	D		Cre	w Chie	f:		Logged OAJ	d By:	
1/21/20	16 2:0	0 pm	25	24.5		9.0	816		3 1/4" H			_			1	J/NJ		



CLIENT/WSE	1	392-010		S	SURFACE EL	EVATION T	: 830.9 f	ft	ı	_	CAR	/PLE	т,	DOT	PA RATO	GE :	
EPTH ELEV. (ft) (ft)		DESCI	RIPTION O	F MATER	IAL	USCS		LOGIC IGIN	N	WL		TYPE			LL (%)		
1		CRUSHED	LIMESTON	VE 0 - 13"			F	ill					(70)	(pcr)	(70)	(70)	
1 + 830		FILL, a mixt	ure of Silty	Sand, Lean	Clay, a few	-	F	ill			1	HSA					
2-829		pieces of Lin	nestone, a fe	ew pieces o	f Wood												
3 - 828									14		2	SB					
4-827																	
5-826																	
6-825									3		3	SB					
7—824																	
ł											4	CD CD					
8 + 823									7		4	SB					
9 + 822																	
10 + 821									4		5	SB					
11 + 820																	
12 — 819		LEAN CLA	Y, gray, we	t, very soft		CL	Fine A	lluvium									
13 — 818									1		6	SB					
14 817																	
15 - 816		FAT CLAY,	dark gray,	saturated, v	ery soft	СН	Fine A	lluvium			_	CD.					
16 - 815									2		7	SB					
17 - 814																	
18 813										]							
19 - 812										⊻							
20 811		TEAN OF A	F7 1 1			CT	Tr' A										
21 — 810		LEAN CLA	Y, dark gray	y, wet, very	soft	CL	Fine A	lluvium	3		8	SB					
22 - 809																	
23 - 808																	
ł																	
24 - 807		SAND WITI water bearing	H A LITTLI g, loose	E GRAVEI	L, brown,	SP	Coarse A	Alluvium									
25 + 806		End of Borin							9	П	9	SB					
26—805		Att v unace	T DATE: A CT	A CITATIA	CNITE			DT - DT	12/2/					4 4 7	17/80	1.6	
DATE T	ME	SAMPLED	LEVEL ME	CAVE-IN	WATER	WA	ΓER	START:		7/201		w Chie	END: f:		17/20 Logge		
	5 pm	DEPTH 25	DEPTH 24.5	DEPTH	DEPTH 18.5	ELEVA 812	ATION	3 1/4" H		- 24	J. T			I	OAJ		
1.17.2010 1.3	- hiii		21.3		10.5	012		J 1/ T 1.	U	<u> </u>	7 100						



-	NT/WSE	1				SURFACE EL	T	1				SAN	/PLE	LA	BOI	RATO	GE ORY	
)EPTH (ft)	ELEV. (ft)		DESCI	RIPTION O	F MATER	IAL	USCS		LOGIC IGIN	N	WL		ТҮРЕ	MC	DD	LL	PL (%)	
1-	- - - 829		FILL, a most of Limestone	ly Sand wit e, brown, da	h Gravel, a rk brown	few pieces		F	ill				HSA	(70)	(pcr)	(70)	(70)	
-	828  827									8		2	SB					
4- 5-	- -826 - -825																	
6- -	- - 824 -									12		3	SB					
-	-823 - -822									9		4	SB					
9- - 10-	-821 - -820		LEAN CLA	Y, gray, mo	ist to wet, s	oft	CL	Fine A	lluvium									
-	- 819 - 818									8	Ā	5	SB					
13 -	- 817 -									5		6	SB					
-	-816 - -815		SAND WITI fine grained,	H A LITTLI brown, wat	E GRAVEI er bearing,	L, medium to loose	SP	Coarse A	Alluvium	6		7	SB					
-	-814 - -813									Ü		,	5.5					
-	- 812 - 811																	
-	- -810 -									7		8	SB					
21 22	809  808 																	
23 –	8 <b>0</b> 7 																	
24-	-806 -		SAND WITI	H A LITTL	E GRAVEI	L, fine to	SP	Coarse	Alluvium									
25-	8 <b>0</b> 5		medium grain End of Borin		ater bearin	g, ioose				9	H	9	SB					
26-	- 8 <b>0</b> 4								T		Ш							
			WATER SAMPLED	LEVEL ME	CAVE-IN	ENTS WATER	XX7 A	TER	START:		1/201		w Chie:	END:		21/20 Logge		
DATE 1/21/20		IME 20 am	DEPTH 25	DEPTH 24.5	DEPTH	DEPTH 12.0	ELEV.	ATION 7.8	METHO 3 1/4" H			J. T	atro			)AJ	. y.	



LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ

### LOG OF TEST BORING

BORING NUMBER PB 13 PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 828 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. **GEOLOGIC** MC DD LL (%) (pcf) (%) DESCRIPTION OF MATERIAL **USCS** Ν LL PL ≶ ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, a mixture of Sand, Sand with Silt, a little Gravel, brown HSA 827 826 825 10 SB 824 823 9 SB 3 822 821 SP SAND WITH A LITTLE GRAVEL, fine to Coarse Alluvium medium grained, brown, wet to water bearing,  $\bar{\Delta}$ 3 820 very loose to loose 4 SB 819 10 818 9 5 SB -817 11 -816 12 7 SB13 -815 6 814 <del>-</del>813 15 SAND WITH A LITTLE GRAVEL, medium to SP Coarse Alluvium 7 SB fine grained, brown, water bearing, loose 16 812 17 811 18 -81019-809 20-808 SANDY LEAN CLAY WITH A LITTLE CLGlacial Till 11 8 SBGRAVEL, gray, wet, firm 807 21 22-806 23 805 804 24 SAND, fine to medium grained, brown, water SP Coarse Alluvium bearing, medium dense 25 803 20 SB End of Boring 25.0 ft. 802 WATER LEVEL MEASUREMENTS START: 11/21/2016 END: 11/21/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/21/2016 1:00 pm 25 24.5 8.0 820 3 1/4" HSA 0' - 24.5 Notes:



BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ

### LOG OF TEST BORING

BORING NUMBER PB 14 PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN CLIENT/WSB #: 03392-010 SURFACE ELEVATION: 825.7 ft PAGE 1 OF 1 LABORATORY TESTS SAMPLE DEPTH ELEV. **GEOLOGIC** MC DD LL PL (%) (pcf) (%) (%) **USCS** DESCRIPTION OF MATERIAL Ν ⋝ ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, a mixture of Sand, Sand with Silt, a little Gravel, brown HSA 825 824 823 16 SB 822 821 20 SB 3 820 819 SAND, fine to medium grained, brown, wet to SP Coarse Alluvium water bearing at 8', loose  $\bar{\Delta}$ 8 818 4 SB 817 -816 7 5 SB -815 11 -814 12 SAND WITH A LITTLE GRAVEL, medium to SP Coarse Alluvium fine grained, brown, water bearing, very loose 2 SB13 -813 -812 811 15 SAND, fine to medium grained, brown, water SP Coarse Alluvium 7 SB bearing, loose to medium dense 16 810 17 809 18 808 -807 19-20-806 20 8 SB805 21 22--804 -803 23 SAND WITH GRAVEL, fine to medium SP Coarse Alluvium grained, brown, water bearing, medium dense -802 24 25 801 SB End of Boring 25.0 ft. 800 WATER LEVEL MEASUREMENTS START: 11/21/2016 END: 11/21/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/21/2016 2:00 pm 25 24.5 8.0 817.7 3 1/4" HSA 0' - 24.5' Notes:



**BORING NUMBER PB 15** PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 825.4 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. **GEOLOGIC** MC DD LL PL (%) (pcf) (%) (%) DESCRIPTION OF MATERIAL USCS Ν ≶ ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, mostly Silty Sand with Gravel, brown, moist HSA 824 823 ORGANIC CLAY, black, a few Roots, moist, OLTopsoil 822 10 SB 821 820 SILTY SAND WITH A LITTLE GRAVEL, SM Coarse Alluvium SB 3 gray, wet, loose -819 818 817 10 SB SAND, fine to medium grained, brown, wet to SP Coarse Alluvium water bearing, loose  $\bar{\Delta}$ 816 -815 SAND WITH GRAVEL, medium to coarse SP Coarse Alluvium 11 5 SB grained, brown, water bearing, medium dense to -814 11 loose to medium dense 12--813 7 SB13 -8126 BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ -811810 15 10 7 SB 16 809 17 808 18 807 19--806 20-805 12 8 SB804 21-22--803 -802 23 --801 24-25 800 SB End of Boring 25.0 ft. 799 WATER LEVEL MEASUREMENTS START: 11/21/2016 END: 11/21/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/21/2016 3:00 pm 25 24.5 8.8 816.6 3 1/4" HSA 0' - 24.5 Notes:



**BORING NUMBER PB 16** PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 828.1 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. **GEOLOGIC** MC DD LL PL (%) (pcf) (%) (%) **USCS DESCRIPTION OF MATERIAL** Ν ≶ ORIGIN (ft) (ft) No. TYPE (%) Fill FILL, mostly Silty Sand, crushed Limstone at grade, gray, brown HSA 827 -826 825 12 SB 824 823 9 3 SB 822 821 SAND WITH A LITTLE GRAVEL, fine to SP Coarse Alluvium 3 820 SB medium grained, brown, wet, very loose to 819 -818 6 5 SB -817 11 12--816 SILTY SAND WITH A LITTLE GRAVEL, SM Coarse Alluvium 7 SB13 -815 gray, wet, loose BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ 814 813 15 SAND WITH A LITTLE GRAVEL, fine to SP Coarse Alluvium  $\bar{\Delta}$ 5 7 SB medium grained, gray, water bearing 15 1/2', 16-812 loose to very loose to loose 17--811 18 810 -80919-20-808 4 8 SB807 21-22--806 23 --805 -80424-25 803 9 SB End of Boring 25.0 ft. 802 WATER LEVEL MEASUREMENTS START: 11/17/2016 END: 11/17/2016 Crew Chief: SAMPLED CASING CAVE-IN Logged By: WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/17/2016 2:45 pm 25 24.5 15.5 812.6 3 1/4" HSA 0' - 24.5' Notes:



	T/WSB	1				SURFACE EL	T	1				SAN	/PLE	1.4	4BOi	RATC	GE ORY	
DEPTH (ft)	ELEV. (ft)		DESCI	RIPTION O	F MATER	IAL	USCS	GEOL ORI	OGIC GIN	N	WL		ТҮРЕ	MC	DD	LL	PL (%)	
+	-826		FILL, mostly little Gravel,	Sand, a litt brown	le Sand wit	th Silt, a		Fi	ill			1	HSA	(70)	(рег)	(70)	(70)	
ł	- 825 - 824									14		2	SB					
+	-823 -822																	
6	821									14		3	SB					
Ŧ	-820 -819									12		4	SB					
9-10-	-818 -817	****	SAND WITI medium grai loose	H A LITTLI ned, brown,	E GRAVEI water bear	L, fine to ring at 9.5',	SP	Coarse A	Alluvium	7	⊻	5	SB					
11 12										,		ر ر	قد ا					
13										8		6	SB					
15	-812		SAND, fine bearing, loos	to medium ;	grained, bro	own, water	SP	Coarse A	Alluvium	5		7	SB					
16	-810																	
18—	- 8 <b>0</b> 8																	
20	- 8 <b>0</b> 6									12		8	SB					
22	-805 -804																	
24																		
+	-802 -801		End of Borin	g 25.0 ft.						14	П	9	SB					
	-		WATER	LEVEL ME	EASUREME	ENTS	1	1	START:	11/2	LL 1/201	.6		END:	: 11/	 21/20	16	<u> </u>
DATE	TI	ME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER DEPTH	WA? ELEVA	TER NETTION			Crew Chief:  J. Tatro				Logged			
1/21/2016 10:30 am 25 24.5 9.5					17.3 3 1/4" HSA 0' - 24.5													



,J	T/WSB					SURFACE EL						SAN	/PLE	T.A	4BOI	PA RATO	)RY	TES'
DEPTH (ft)	ELEV. (ft)		DESCI	RIPTION O	F MATER	IAL	USCS	GEOL ORIO		N	WL		TYPE	MC	DD	LL	PL (%)	
+	- 825		FILL, mostly Limstone, br	Silty Sand, own	, a few piec	es of		Fil	II			1	HSA	(70)	(poi)	(70)	(70)	
2	- -824																	
+	-823									12		2	SB					
ł	-822 - -821																	
ł	- -820									9		3	SB					
ł	- -819 -		SAND WITI medium grai	H A LITTLI	E GRAVEI , wet to wat	_, fine to	SP	Coarse A	lluvium									
ł	-818 - -817		11.5', loose			_				6		4	SB					
10	- -816 -									7		5	SB					
11	-									,	ӯ							
12-	-									7		6	SB					
14	- -812 -		SILTY SAN	D WITH C	DANEL	any trotat	SM	Coarse A	House diverses									
15— 16—	-		bearing, med	ium dense	KAVEL, gi	ray, water	SIM	Coarse A	.IIUVIUIII	9		7	SB					
17																		
18	-																	
19—																		
21	- -805 -									16		8	SB					
22-	-804 - -803																	
ł	-802																	
25	-		End of Borin	g 25.0 ft.						14	H	9	SB					
26	-800										Ш							
				LEVEL ME	1		TTTAT	LED	START:		8/201			END:				
DATE         TIME         SAMPLED DEPTH         CASING DEPTH         CAVE-IN DEPTH         WATER DEPTH           1/18/2016         2:30 pm         25         24.5         11.5					ELEVA	TER ATION         METHOD         Crew Chief:         Logger           J. Tatro         DAJ           4.4         3 1/4" HSA 0' - 24.5' Notes:			. y .									



BORING LOG - WSB.GDT - 12/7/16 08:00 - K:\03392-010\GEOTECH-CMT\HCPW - 6701 W 78TH ST, BLOOMINGTON MN.GPJ

### LOG OF TEST BORING

BORING NUMBER PB 19 PROJECT NAME: HCPW - 6701 W 78th Street PROJECT LOCATION: Bloomington, MN SURFACE ELEVATION: 824.3 ft CLIENT/WSB #: 03392-010 PAGE 1 OF 1 LABORATORY TESTS DEPTH ELEV. **GEOLOGIC** MC DD LL PL (%) (pcf) (%) DESCRIPTION OF MATERIAL **USCS** N ≶ ORIGIN (ft) (ft) No. TYPE Fill FILL, a mixture of Sand, Silty Sand, a little Gravel, a few pieces of Limestone 823 **HSA** 822 821 8 SB 820 -819 10 SB 3 818 817 816 12 4 SB 815 SAND WITH GRAVEL, fine to medium SP Coarse Alluvium grained, brown, wet, medium dense to very 10 814 12 5 SB -813 11 12 -812 SB13 -811 12 6 810 -809 15 4 7 SB 16 808 17 807 18 806 19-805 SAND WITH GRAVEL, fine to medium SP Coarse Alluvium grained, gray, wet to water bearing at 22', loose 20-804 5 8 SB803 21- $\nabla$ 22--802 23 -801 800 24 SILTY SAND, brown, water bearing, loose SP Coarse Alluvium 25 799 SB End of Boring 25.0 ft. 798 WATER LEVEL MEASUREMENTS START: 11/18/2016 END: 11/18/2016 Crew Chief: SAMPLED CASING Logged By: CAVE-IN WATER WATER DATE TIME **METHOD** DEPTH DEPTH **DEPTH** DEPTH ELEVATION J. Tatro DAJ 11/18/2016 1:45 pm 25 24.5 22.0 802.3 3 1/4" HSA 0' - 24.5 Notes:



1	NT/WSB					SURFACE EL	T	1				SAN	/PLE	L	ABOI	PA RATO		
(ft)	ELEV. (ft)		DESCI	RIPTION O	F MATER	IAL	USCS	GEOL ORI	OGIC GIN	N	WL		TYPE	MC	DD	LL	PL (%)	
	- -825		FILL, a mixt Gravel, a few brown	ure of Silty pieces of I	Sand, Sand Limestone,	l, a little dark brown,		Fi	iШ			1	HSA	(70)	(poi)	(70)	(70)	
-	-824 - -823									8		2	SB					
4-	- -822 -																	
_	821  820 									4		3	SB					
-	-819 - -818		LEAN CLA	Y, gray, we	t, very soft		CL	Fine Al	lluvium	2		4	SB	21	91			
-	- 817 - 816																	
_	816  815									2		5	SB					
-	-814 - -813									2		6	SB					
-	- -812 -		FAT CLAY,	gray, wet,	soft		СН	Fine Al	lluvium									
_	-811 - -810 -									6		7	SB					
-	809  808										Δ							
-	- -807 -		SILTY SAN gray, water b	D WITH A	LITTLE G	RAVEL,	SP	Coarse A	Alluvium									
-	806 - 805 -									5		8	SB					
22- 23-	-804 - -803																	
-	- -802 - -801									8.		9	SB					
26-	-800 -800		End of Borin	g 25.0 ft.						O		9	30					
				LEVEL ME					START:	11/1	8/201					18/20		
DATE		ME 5 am	SAMPLED DEPTH 25	CASING DEPTH 24.5	CAVE-IN DEPTH	WATER DEPTH 18.0	WAT ELEVA	ATER METHOD  07.7 3 1/4" HSA 0' - 24			Crew Chief: J. Tatro				Logged By: DAJ			



CLIEN	VT/WSB	#: 03	392-010	W 78th Stre		PROJECT LOG SURFACE EL									UM	PA	GE 1	1 OI
	ELEV.		DESCI	RIPTION C	F MATER	IAL	USCS		OGIC GIN	N	WL		IPLE TYPE			LL (%)		res <sup>-</sup>
(ft)	(ft)		FILL, a mixt	ure of Sand	, Silty Sand	I, Sand with			ill			No.	TYPE	(%)	(pcf)	(%)	(%)	
1	- 824 		Silt, a little C brown	Gravel, a fev	v pieces of l	Limestone,						1	HSA					
2	-823																	
3	- 822									13		2	SB					
4-	- 821																	
5-	- 820																	
6-	- 819									4		3	SB					
7	_																	
+	_		FAT CLAY,	dark gray,	wet, very so	oft	СН	Fine A	lluvium	2			65	]	0.5			
8-	_									3		4	SB	23	95			
9-	-816 -																	
10	-815 -		SAND WITI gray, water b	H SILT, fine	e to mediun	n grained,	SP-SM	Coarse A	Alluvium	7		5	SB					
11	-814 -		gray, water e	caring, 100s	,						Δ							
12	-813 -		FAT CLAY,	dark gray,	wet, firm		СН	Fine A	lluvium									
13	-812									10		6	SB					
14	-811																	
15	- 810		SAND WITI grained, gray	H GRAVEI	., fine to me ring, loose	edium	SP	Coarse A	Alluvium									
16	- 809									8		7	SB					
17	- 8 <b>0</b> 8																	
18-	- 8 <b>0</b> 7																	
19-	- 806																	
20	- 805																	
+	- 804									10		8	SB					
22	- -803																	
23	_																	
+	_																	
24	_		SAND WITI water bearing	H GRAVEI g, loose	., fine grain	ed, brown,	SP	Coarse A	Alluvium	_								
25	_		End of Borin							_ 7	П	9	SB					
26	<del>- 799</del>		HI A TEN	TEMEN NO	CACITO EXP	ENITE			ET A D.T.	1 1 /1	2/201		L	ENTE	4 4 7	1.0/50	1.6	
DATE		ME	SAMPLED	LEVEL ME	CAVE-IN	WATER	WA	TER	START:		5/201		w Chie	END: f:		18/20 _ogge		_
DATE		ME 15 am	DEPTH 25	DEPTH 24.5	DEPTH	DEPTH 11.0	ELEVA 814	ATION METHOD J. Tatro DAJ										
DATE         TIME         SAMPLED DEPTH         CASING DEPTH         CAVE-IN DEPTH         WATER DEPTH           11/18/2016         11:45 am         25         24.5         11.0				2-1.3	817		J 1/T 11	D/ I U	- 47.	7 '''	· 3.							



	ELEV.	05	392-010			URFACE EL	<u> </u>					SAN	/PLE	L	ABOI	RATO	GE DRY	
(ft)	ELEV.		DESCI	RIPTION O	F MATER	IAL	USCS	GEOL ORIG		N	WL		TYPE	MC	DD	LL	PL (%)	
+	- -823 - -822		FILL, a mixt little Gravel,	ure of Sand a few piece	with Silt, S s of Limest	Silty Sand, a one		Fi	11			1	HSA	(78)	(per)	(78)	(78)	
3-	- -821 - -820									16		2	SB					
	- 819 - 818		SAND WITI GRAVEL, b			Æ	SP-SM	Coarse A	lluvium	8		3	SB					
8- -	817  816 									8		4	SB					
-	-815 - -814 -		LEAN CLA	Y, dark gray	y, wet, soft		CL	Fine Al	luvium	6		5	SB					
-	-813 - -812		FAT CLAY,	dark gray,	wet to satur	rated at 19',	СН	Fine All	luvium									
-	- -811 - -810		very soft							2		6	SB					
15 – 16 –	- 809 - 808											7	SB					
17 <del>-</del> -	- 8 <b>0</b> 7 -																	
18- - 19-	806  805 										Ā							
20 — 21 —	-804 - -803									2		8	SB					
22 — 23 —	-802 - -801																	
24 – 25 –	- 800 - 799		End of Borin	og 25 0 #						2		9	SB					
26-	- 798		ENG OF ROLL	ıg 23.0 II.														
			WATER	LEVEL ME	EASUREME	ENTS	1	<u> </u>	START:	11/1	LЦ 8/201	.6		END:	: 11/	18/20	16	
DATE TRUE SAMPLED CASING CAVE-IN WATER						WAT ELEVA	ER TION	I METHOD -			Crew Chief:			I	Logged By:			
18—806 19—805 20—804 21—803 22—802 23—801 24—800 25—799 End of Boring 25.0 ft.  WATER LEVEL MEASUREMENTS  DATE TIME SAMPLED DEPTH DEPTH DEPTH  11/18/2016 12:30 pm 25 24.5 19.0					805		3 1/4" HSA 0' - 24.5 Notes:											



### SYMBOLS AND TERMINOLOGOY ON TEST BORING LOG

	SYMBOL	.S	
	Drilling and Sampling		Laboratory Testing
Symbol	Description	Symbol	<u>Description</u>
HSA _FA _HA _DC _RC PD CS DM JW SB _L _T _TT _TO W B P _Q _X N CR WL ▼ NMR	3-1/4" LD. Hollow stem auger 4", 6" or 10" diameter flight auger 2", 4", or 6" hand auger 2-1/2", 4", 5", or 6" steel drive casing Size A, B or N rotary casing Pipe drill or cleanout tube Continuous split barrel sampling Drilling mud Jetting water 2" O.D. split barrel sampling 2-1/2" or 3-1/2" O.D. SB liner sampler 2" or 3" thin walled tube sample 3" thin walled tube using pitcher sampler 2" or 3" thin walled tube using Osterberg sampler Wash sample Bag sample Test pit sample BQ, NQ, or PQ wire line system AX, BX, or NX double tube barrel Standard penetration test, blows per foot Core recovery, percent Water level Water level No measurement recorded, primarily due to presence of drilling or coring fluid.		Water content, % (ASTM** D2216) Dry density, pcf Liquid limit (ASTM D4318) Plastic limit (ASTM D4318)  -Inserts in last column (Qu or RQD)-  Unconfined compressive strength, psf (ASTM D2166) Penetrometer reading, tsf (ASTM D1558) Torvane reading, tsf (SSTM D1558) Torvane reading, tsf Specific gravity (ASTM D854) Shrinkage limits (ASTM D427) Organic content-combustion method (ASTM D2974) Swell pressure, tsf (ASTM D4546) Percent swell under pressure (ASTM D4546) Free swell, % (ASTM D4546) Shrink swell, % (ASTM D4546) Hydrogen ion content-Meter Method (ASTM D4972) Sulfate content, parts/million or mg/l Chloride content, parts/million or mg/l One dimensional consolidation (ASTM D2435) Triaxial compression (ASSTM D2850 and D4767) Direct Shear (ASTM D3080) Coefficient of permeability, cm/sec (ASTM D2434) Pinhole test (ASTM D4647) Double hydrometer (ASTM D4221) Particle size analysis (ASTM D422) Laboratory electrical resistivity, ohm-cm (ASTM G57) Pressuremeter deformation modulus, tsf (ASTM D4719) Pressuremeter test (ASTM D3385) Rock quality designation, percent s shown on attached data sheet or graph M designates American Society for Testing and Materials

			TERMIN	OLOGY						
	Particl	e Sizes			Soil layering a	ınd Moisture				
Type Boulders Cobbles Coarse gra Fine grave Coarse san Medium sa Fine sand Silt Clay	1 #4 sieve - 3 d #4 - #10 sie and #10-#40 sie #40-#200 sie 100% passin	ve ve eve ig #200 sieve a	and > 0.005mm and < 0.005mm	Term Lamination Varved  Lenses Stratified Layer Dry Moist Waterbeari Wet	Altering lamination clay, silt, fine sand, Small pockets of di Altering layers of v 1/4" to 12" thick st Powdery, no notice Damp, below satur	atum s of any comb or colors fferent soils in arying materia ratum able water ation v water	a soil mass			
	Gravel	Content		Standard Penetration Resistance						
Coars	se-Grained Soils	Fine-	Grained Soils	Coh	esionless Soils	Cohe	esive Soils			
% Gravel 2-15 16-49	<u>Description</u> A little gravel With gravel	% Gravel < 5 5-15 16-30 31-49	Description Trace of gravel A little gravel With gravel Gravelly	N-Value 0-4 5-10 11-30 31-50 > 50	Relative Density Very loose Loose Medium dense Dense Very dense	N-Value 0-4 5-8 9-15 16-30 > 30	Consistency Very soft Soft Firm Hard Very hard			



#### NOTICE TO REPORT USERS BORING LOG INFORMATION

#### Subsurface Profiles

The subsurface stratification lines on the graphic representation of the test borings show an approximate boundary between soil types or rock. The transition between materials is approximate and is usually far more gradual than shown. Estimating excavation depths, soil volumes and other computations relying on the subsurface strata may not be possible to any degree of accuracy.

#### Water Level

WSB & Associates, Inc. took groundwater level readings in the exploratory borings, reviewed the data obtained, and discussed its interpretation of the data in the text of this report. The groundwater level may fluctuate due to seasonal variations caused by precipitation, snowmelt, rainfalls, construction or remediation activities, and/or other factors not evident at the time of measurement.

The actual determination of the subsurface water level is an interpretative process. Subsurface water level may not be accurately depicted by the levels indicated on the boring logs. Normally, a subsurface exploration obtains general information regarding subsurface features for design purposes. An accurate determination of subsurface water levels is not possible with a typical scope of work. The use of the subsurface water level information provided for estimating purposes or other site review can present a moderate to high risk of error.

The following information is obtained in the field and noted under "Water Level Measurements" at the bottom of the log.

Sampled Depth: The lowest depth of soil sampling at the time a water level measurement is taken.

Casing Depth: The depth to the bottom of the casing or hollow-stem auger at the time of water

level measurement.

Cave-In Depth: The depth at which the measuring tape stops in the bore hole.

Water Level: The point in the bore hole at which free-standing water is encountered by a

measuring tape dropped from the surface inside the casing.

Drilling Fluid Level: Similar to the water level, except the liquid in the bore hole is a drilling fluid.

### Obstruction Depths

Obstruction and/or obstruction depths may be noted on the boring logs. Obstruction indicates the sampling equipment encountered resistance to penetration. It must be realized that continuation of drilling, the use of other drilling equipment or further exploration may provide information other than that depicted on the logs. The correlation of obstruction depths on the log with construction features such as rock excavation, foundation depths, or buried debris cannot normally be determined with any degree of accuracy. For example, penetration of weathered rock by soil sampling equipment may not correlate with removal by certain types of construction equipment. Using this information for estimating purposes often results in a high degree of misinterpretation.

Accurately identifying the obstruction or estimating depths where hard rock is present over the site requires a scope of service beyond the normal geotechnical exploration program. The risk of using the information noted on the boring logs for estimating purposes must be understood.



# UNIFIED SOIL CLASSIFICATION SYSTEM

UNIFIED SOI	LCLASS	IFICATION AND SYMBOL CHART
		RSE-GRAINED SOILS
(more than	50% of mat	terial is larger than No. 200 sleve size.)
	Clean	Gravels (Less than 5% fines)
GRAVELS	GW	Well-graded gravels, gravel-sand mixtures, little or no fines
More than 50% of coarse	GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines
fraction larger than No. 4	Grave	ls with fines (More than 12% fines)
sieve size	GM	Silty gravels, gravel-sand-silt mixtures
	GC	Clayey gravels, gravel-sand-clay mixtures
	Clean	Sands (Less than 5% fines)
SANDS	sw	Well-graded sands, gravelly sands, little or no fines
50% or more of coarse	SP	Poorly graded sands, gravelly sands, little or no fines
fraction smaller	Sands	with fines (More than 12% fines)
than No. 4 sieve size	SM	Silty sands, sand-silt mixtures
	sc	Clayey sands, sand-clay mixtures
PRODUCTION AND A STATE OF THE S	FINE	-GRAINED SOILS
(50% or m	ore of mate	rial is smaller than No. 200 sieve size.)
SILTS AND	ML	inorganic silts and very fine sands, rock flour, silty of clayey fine sands or clayey silts with slight plasticity
CLAYS Liquid limit less than	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
50%	OL	Organic silts and organic silty clays of low plasticity
SILTS AND	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
CLAYS Liquid limit 50%	сн	Inorganic clays of high plasticity, fat clays
or greater	ОН	Organic clays of medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS	24 2 2 PT 24	Peat and other highly organic soils

25yuuuwaaaa	LABORATORY CLAS	SIFICATION CRITERIA								
GW	$C_{tt} = \frac{D_{60}}{D_{10}}$ greater than	4; $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3								
GP	Not meeting all gradation requirements for GW									
GM	Atterberg limits below "A" Above "A" line with P.I. between 4 and 7 are borderline cases									
GC	Atterberg limits above "A" line with P.I. greater than 7	"A" requiring use of dual symbols								
sw	$C_u = \frac{D_{60}}{D_{10}}$ greater than	4; $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3								
SP	Not meeting all gradation requirements for GW									
SM	Atterberg limits below "A" Limits plotting in shaded zon- with P.I. between 4 and 7 are									
sc	Atterberg limits above "A" line with P.I. greater than 7	borderline cases requiring use of dual symbols.								

