



STORMWATER MANAGEMENT PLAN
FOR
VALLEY WEST SHOPPING CENTER
10520 FRANCE AVE S
BLOOMINGTON, MN

PREPARED BY:
JOSH PISTORIUS, EIT
BRADY BUSSELMAN, PE

PROJECT INTRODUCTION

The proposed project is a 0.88-acre redevelopment located at the Valley West Shopping Center located northwest of the intersection of W Old Shakopee Rd and France Ave S in Bloomington, MN. The project is located within the Lower Minnesota River Watershed District. Stormwater management onsite has been designed to meet the rate control and volume retention requirements of the City and Watershed. The City of Bloomington is the LGU.

EXISTING CONDITIONS

The existing project area is a building and parking lot. The project area to be disturbed is 0.88 acres, with 0.81 acres of impervious area (92% impervious). Geotechnical borings prepared by Braun Intertec show under lying soils to be poorly graded sands, which may be classified as Hydrologic Soil Group type A soils and are highly suitable for infiltration. The site drains into the adjacent storm sewer which travels north. There is currently no stormwater management system in place.

PROPOSED CONDITIONS

The proposed project will reconstruct the building and parking lot areas, with a reduced impervious area from 0.81 acres to 0.69 acres of new & reconstructed impervious (percentage impervious reduced to 78%). In order to provide stormwater management onsite, an underground infiltration system is proposed beneath the parking lot. The system will discharge to the existing storm sewer to the east. The subsurface system is designed only to meet volume and water quality. The reduction in impervious area alone is sufficient to meet rate control requirements, as demonstrated in the attached HydroCAD calculations. It should be noted that due to the lack of existing storm sewer in the area of the project, and due to the spread out nature of disturbed area, it is proposed to construct a new roof drain pipe for the 13,044 SF existing building roof. This roof is not part of the disturbed area, but when combined with the surface runoff captured south of the building, it routes a comparable amount of impervious area to the system.

Disturbed Area vs. Area Routed to Subsurface System (Acres)

	Disturbed Area (Existing)	Disturbed Area (Proposed)	Area Routed to Subsurface System
<i>Pervious Area</i>	0.07	0.19	0.10
<i>Impervious Area</i>	0.81	0.69	0.85
<u><i>Total</i></u>	<u>0.88</u>	<u>0.88</u>	<u>0.95</u>

RATE CONTROL

The City of Bloomington requires that the redevelopment must achieve a net reduction of pre-project discharge rates for the 2-, 10-, and 100-year 24-hour Atlas 14 rainfall events. Rate control is achieved onsite through the reduction of onsite impervious surface, as noted above and demonstrated in the attached HydroCAD calculations. The runoff rate control analysis was performed in HydroCAD using the MSE 3 rainfall distribution. The results of the analysis follow with additional information available in the appendices.

Maximum Rate of Runoff (cfs)		
Storm Event	Total Existing	Total Proposed
<i>2-year</i>	3.23	2.34
<i>10-year</i>	5.17	4.26
<i>100-year</i>	9.58	8.78

VOLUME REDUCTION

The City of Bloomington requires onsite retention of the first 1.1 inches of runoff from new and reconstructed impervious surfaces. The proposed storm sewer onsite drains to an underground infiltration system which captures the first 1.1 inches of rainfall and bypasses the larger storm events. The system consists of 6 inches of bottom rock, a 30-inch chamber, and another 6 inches of top rock, for a total depth of 3.75 feet. The rock within the system has 40% voids, reducing the equivalent depth of the system to 2.9 feet. Based on the underlying HSG type A soils, an infiltration rate of 0.8 in/hr and maximum infiltration depth of 3.2 feet are used for the design. The results of the volume retention calculations are provided below.

$$\text{Required Infiltration Volume}(ft^3) = V_{inf} = 1(in) * \frac{1 ft}{12 in} * \text{New Impervious Area}(ft^2)$$

$$V_{inf}(ft^3) = 1.1(in) * \frac{1 ft}{12 in} * 30,056 (ft^2) = 2,755 ft^3$$

Volume Control Analysis		
New Impervious Surface	30,056	sf
Design Infiltration/Filtration Rainfall Event	1.1	in
Required Infiltration/Filtration Volume	2,755	cf
Maximum Allowable Infiltration Rate	0.80	in/hr
Required Drawdown Time	48	hrs
Maximum Live Storage Depth	3.2	ft
Provided Volume Below Outlet	2,764	cf

WATER QUALITY

The City of Bloomington water quality requirements vs. the proposed system are shown in the table below. MIDS calculator results have been attached to this SWMP.

Water Quality – Removal Requirements vs. Actual (%)		
	Minimum Required	Provided
<i>TSS</i>	90	97
<i>TP</i>	60	97

STORMWATER SYSTEM OPERATIONS & MAINTENANCE

An operations & maintenance agreement has been prepared for the project, and is attached to this SWMP.

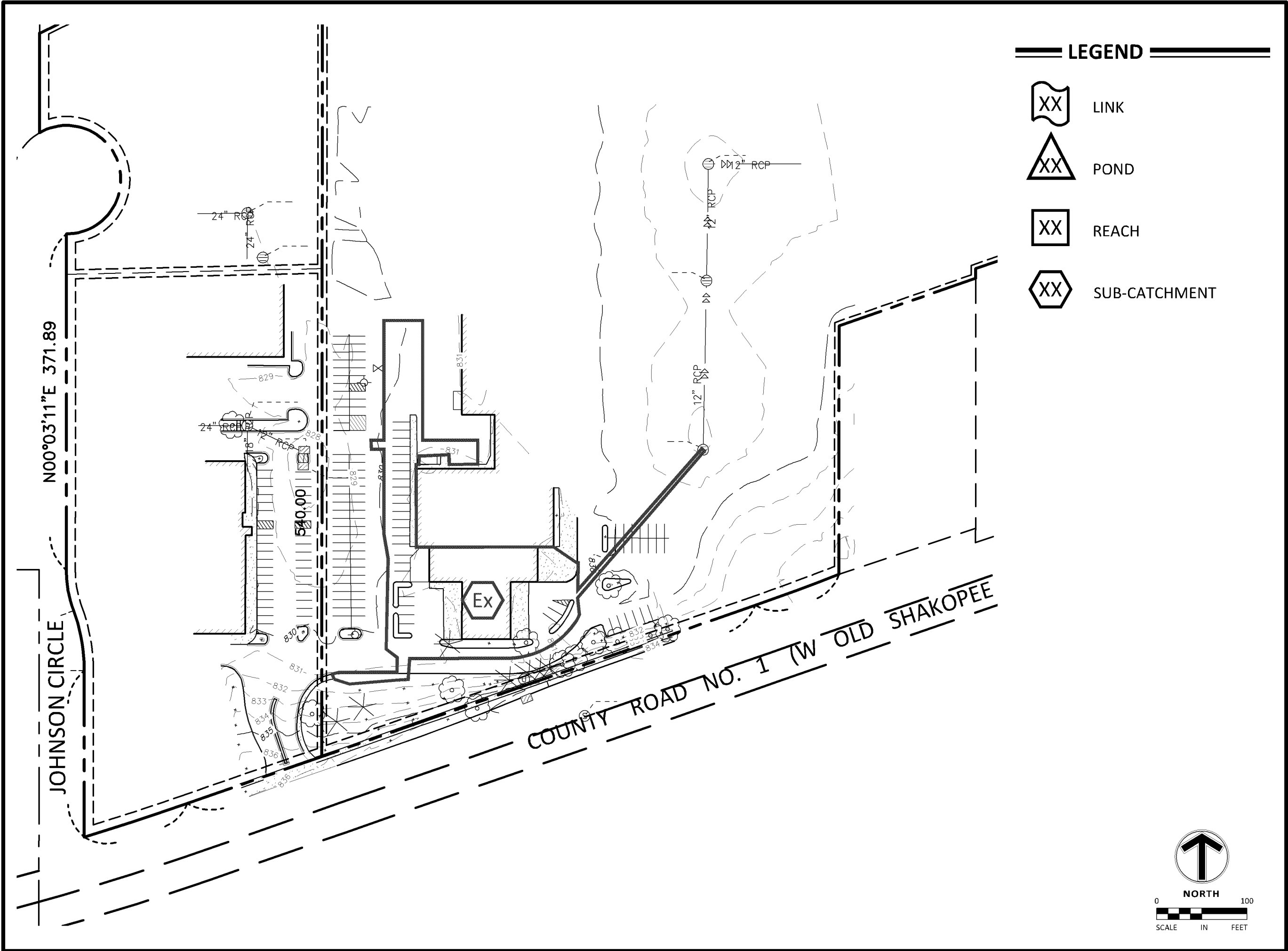
SUMMARY

The proposed Valley West Shopping Center project will meet the requirements of the City of Bloomington, Lower Minnesota River Watershed District, and MPCA through construction of an underground infiltration system and through reduction of impervious area. This BMP will provide the required volume reduction and water quality removals prior to discharging stormwater runoff from the site to downstream receiving waters.

If you have any questions, comments, or additional information regarding this report, please contact me at bbusselman@sambatek.com or 763-259-6674.



APPENDIX A – DRAINAGE MAPS



Client

**KRAUS
ANDERSON**
Project
VALLEY WEST

Location
**BLOOMINGTON,
MN**

Certification

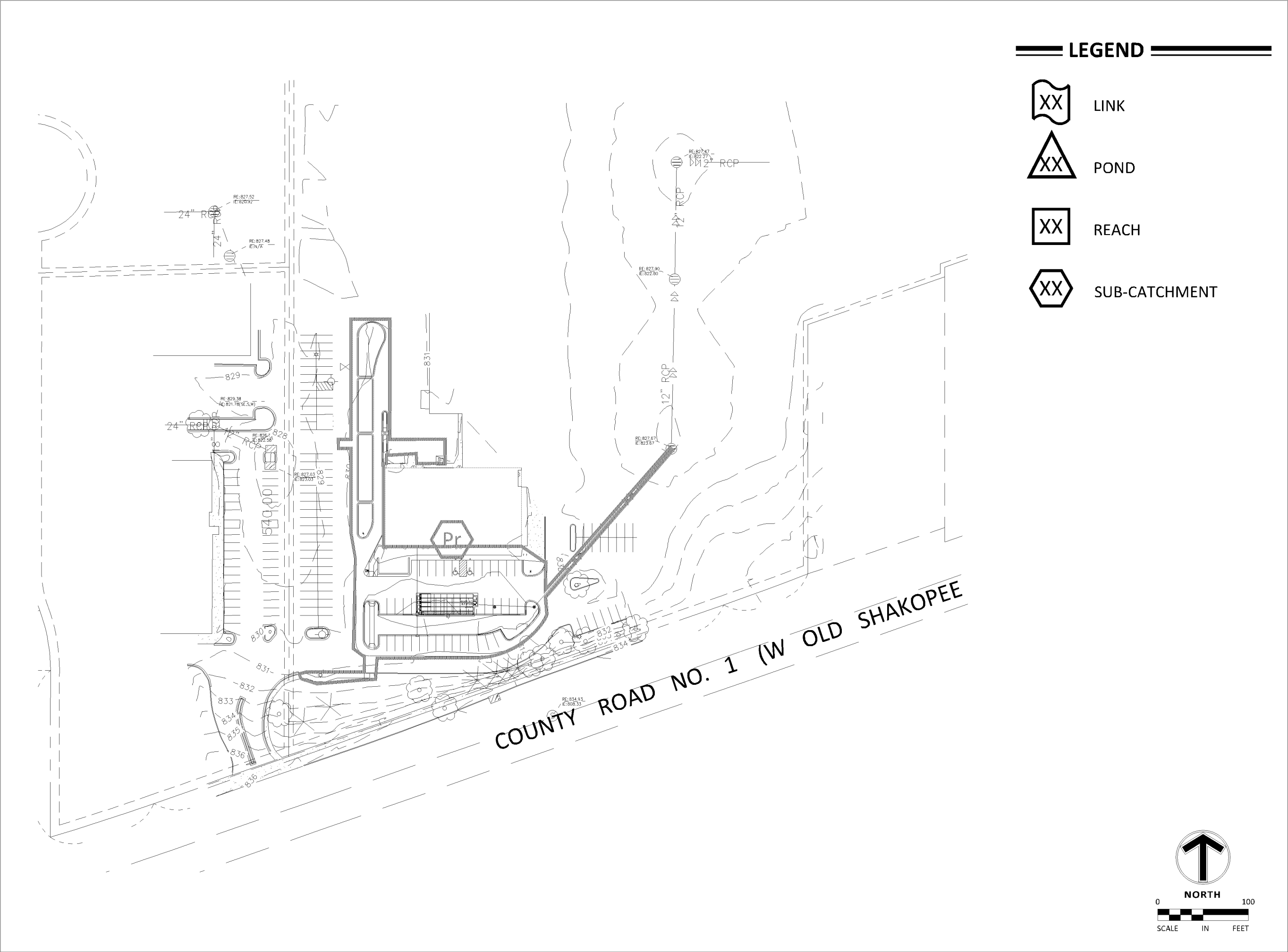
Summary
Approved: BB Drawn: AJR

Revision History
No. Date By Submittal / Rev.

Sheet Title
EXISTING
DRAINAGE MAP
- LIMITS OF
DISTURBANCE

Sheet No. Revision
1/3

Project No. 21618



Client

**KRAUS
ANDERSON**

**Project
VALLEY WEST**

**Location
BLOOMINGTON,
MN**

Certification

Summary
Approved: BB Drawn: AJR

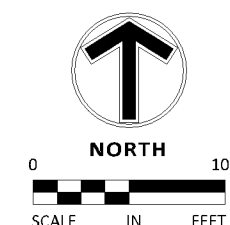
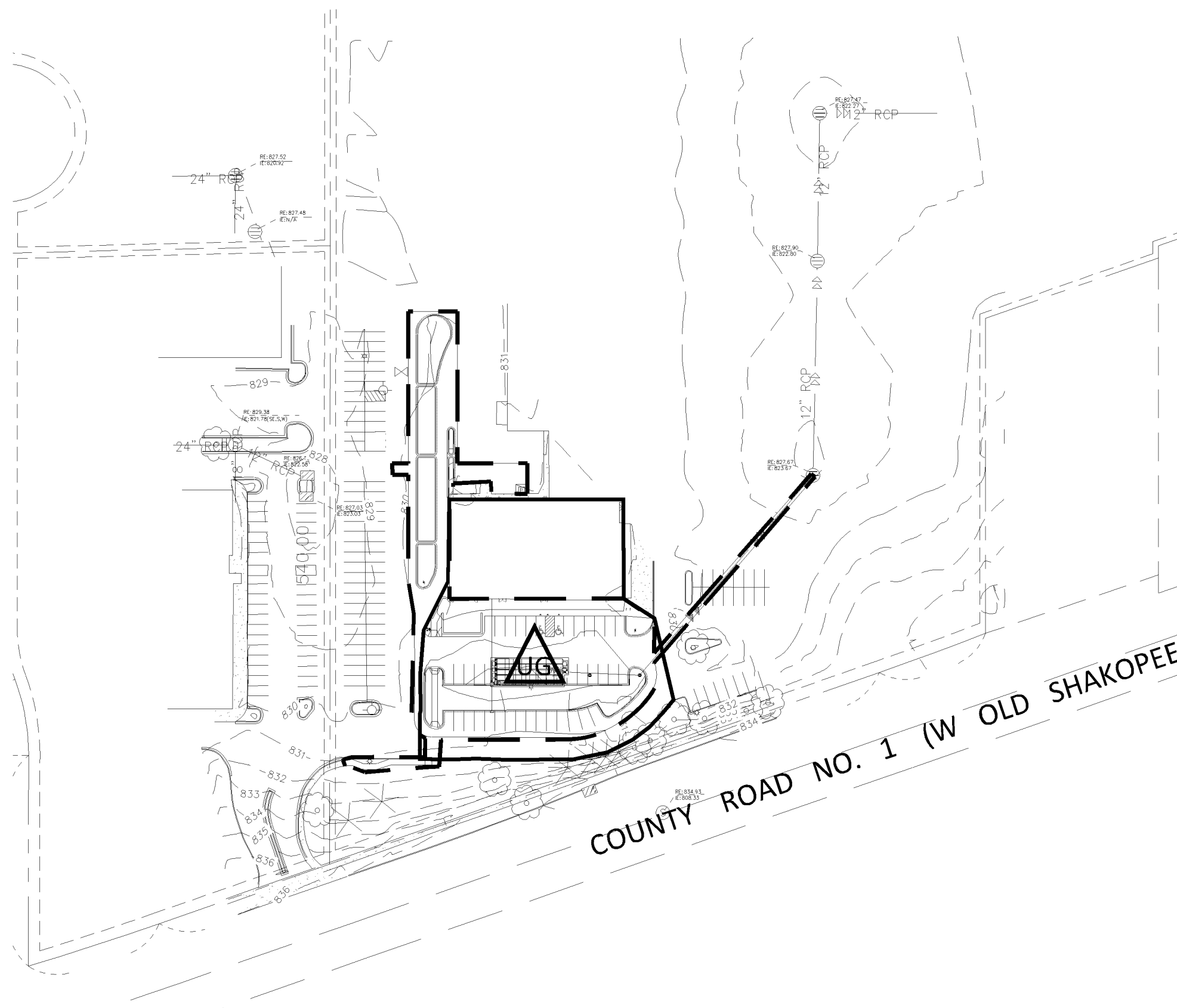
Revision History
No. Date By Submittal / Rev.

Sheet Title
PROPOSED
DRAINAGE MAP
- LIMITS OF
DISTURBANCE

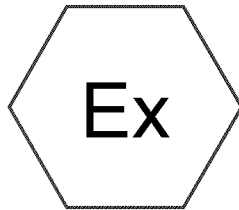
Sheet No. Revision
2/3

Project No. 21618

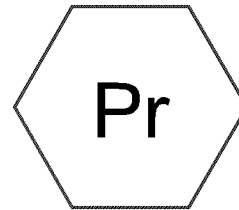
Project No. 21618



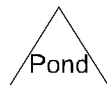
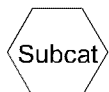
APPENDIX B – HYDROCAD CALCULATIONS



Existing Area to be
Disturbed



Proposed Area to be
Redeveloped



Routing Diagram for 21618 - HydroCAD - SC740

Prepared by MFRA, Printed 12/17/2020

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Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.260	39	>75% Grass cover, Good, HSG A (Ex, Pr)
0.810	98	Paved parking, HSG A (Ex)
0.690	98	Paved parking, HSG C (Pr)
1.760	89	TOTAL AREA

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Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
1.070	HSG A	Ex, Pr
0.000	HSG B	
0.690	HSG C	Pr
0.000	HSG D	
0.000	Other	
1.760		TOTAL AREA

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Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.260	0.000	0.000	0.000	0.000	0.260	>75% Grass cover, Good	Ex, Pr
0.810	0.000	0.690	0.000	0.000	1.500	Paved parking	Ex, Pr
1.070	0.000	0.690	0.000	0.000	1.760	TOTAL AREA	

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MSE 24-hr 3 2-Year Rainfall=2.83"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Ex: Existing Area to be Runoff Area=0.880 ac 92.05% Impervious Runoff Depth=2.09"
Tc=5.0 min CN=93 Runoff=3.23 cfs 0.153 af

Subcatchment Pr: Proposed Area to be Runoff Area=0.880 ac 78.41% Impervious Runoff Depth=1.45"
Tc=5.0 min CN=85 Runoff=2.34 cfs 0.106 af

Total Runoff Area = 1.760 ac Runoff Volume = 0.259 af Average Runoff Depth = 1.77"
14.77% Pervious = 0.260 ac 85.23% Impervious = 1.500 ac

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MSE 24-hr 3 2-Year Rainfall=2.83"

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Summary for Subcatchment Ex: Existing Area to be Disturbed[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 3.23 cfs @ 12.11 hrs, Volume= 0.153 af, Depth= 2.09"

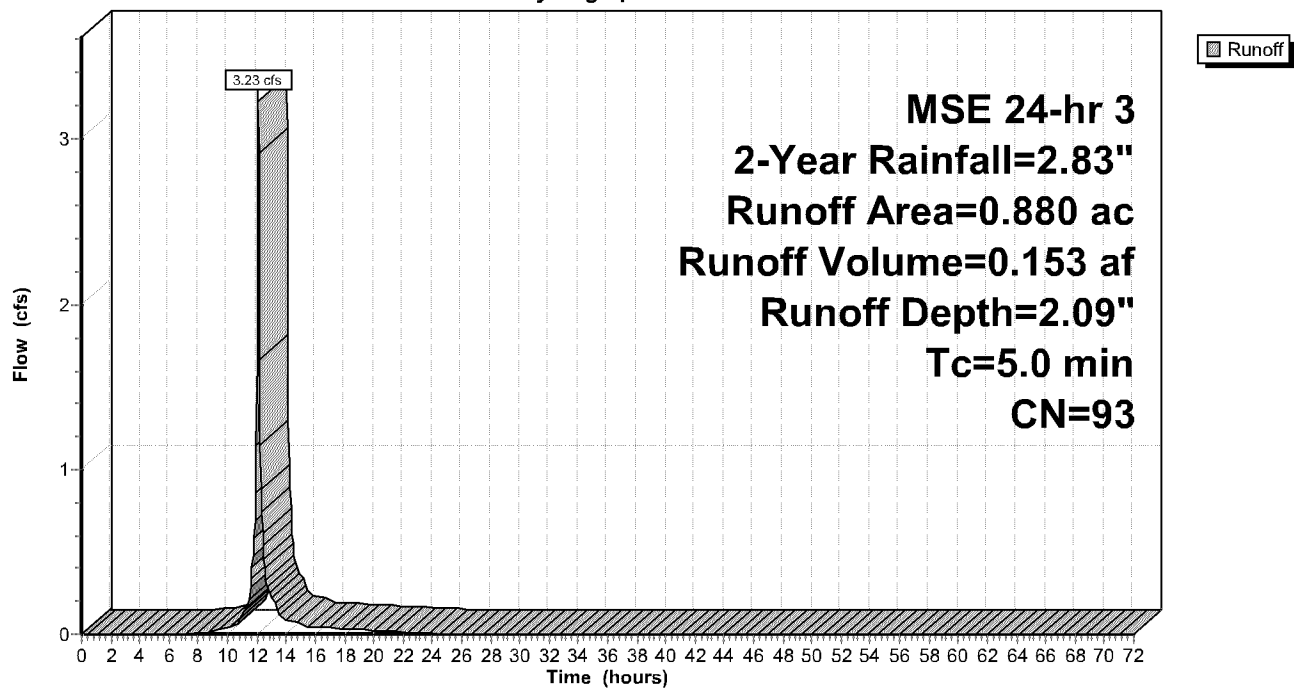
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, $dt=0.05$ hrs
MSE 24-hr 3 2-Year Rainfall=2.83"

Area (ac)	CN	Description
0.810	98	Paved parking, HSG A
0.070	39	>75% Grass cover, Good, HSG A
0.880	93	Weighted Average
0.070		7.95% Pervious Area
0.810		92.05% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment Ex: Existing Area to be Disturbed

Hydrograph



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MSE 24-hr 3 2-Year Rainfall=2.83"

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Summary for Subcatchment Pr: Proposed Area to be Redeveloped[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 2.34 cfs @ 12.12 hrs, Volume= 0.106 af, Depth= 1.45"

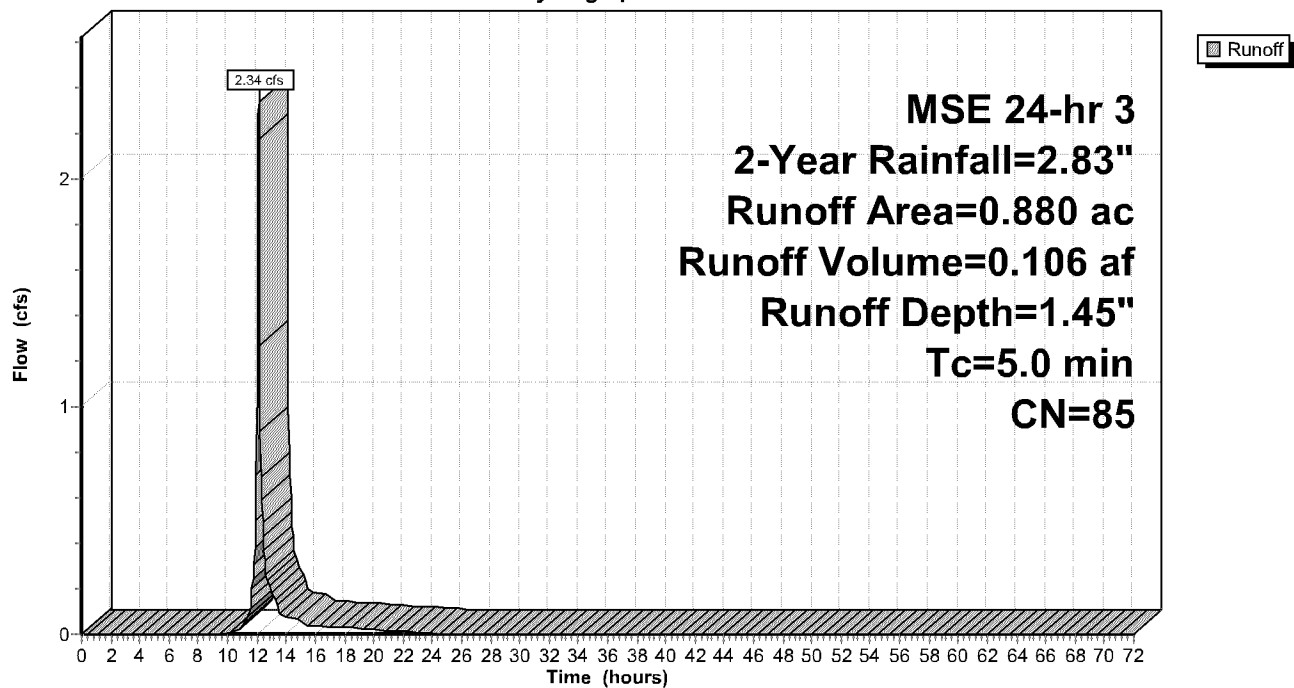
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, $dt=0.05$ hrs
MSE 24-hr 3 2-Year Rainfall=2.83"

Area (ac)	CN	Description
0.690	98	Paved parking, HSG C
0.190	39	>75% Grass cover, Good, HSG A
0.880	85	Weighted Average
0.190		21.59% Pervious Area
0.690		78.41% Impervious Area

T_c (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment Pr: Proposed Area to be Redeveloped

Hydrograph



21618 - HydroCAD - SC740

MSE 24-hr 3 10-Year Rainfall=4.24"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Ex: Existing Area to be Runoff Area=0.880 ac 92.05% Impervious Runoff Depth=3.45"
Tc=5.0 min CN=93 Runoff=5.17 cfs 0.253 af

Subcatchment Pr: Proposed Area to be Runoff Area=0.880 ac 78.41% Impervious Runoff Depth=2.67"
Tc=5.0 min CN=85 Runoff=4.26 cfs 0.196 af

Total Runoff Area = 1.760 ac Runoff Volume = 0.449 af Average Runoff Depth = 3.06"
14.77% Pervious = 0.260 ac 85.23% Impervious = 1.500 ac

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MSE 24-hr 3 10-Year Rainfall=4.24"

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Summary for Subcatchment Ex: Existing Area to be Distrurbed[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 5.17 cfs @ 12.11 hrs, Volume= 0.253 af, Depth= 3.45"

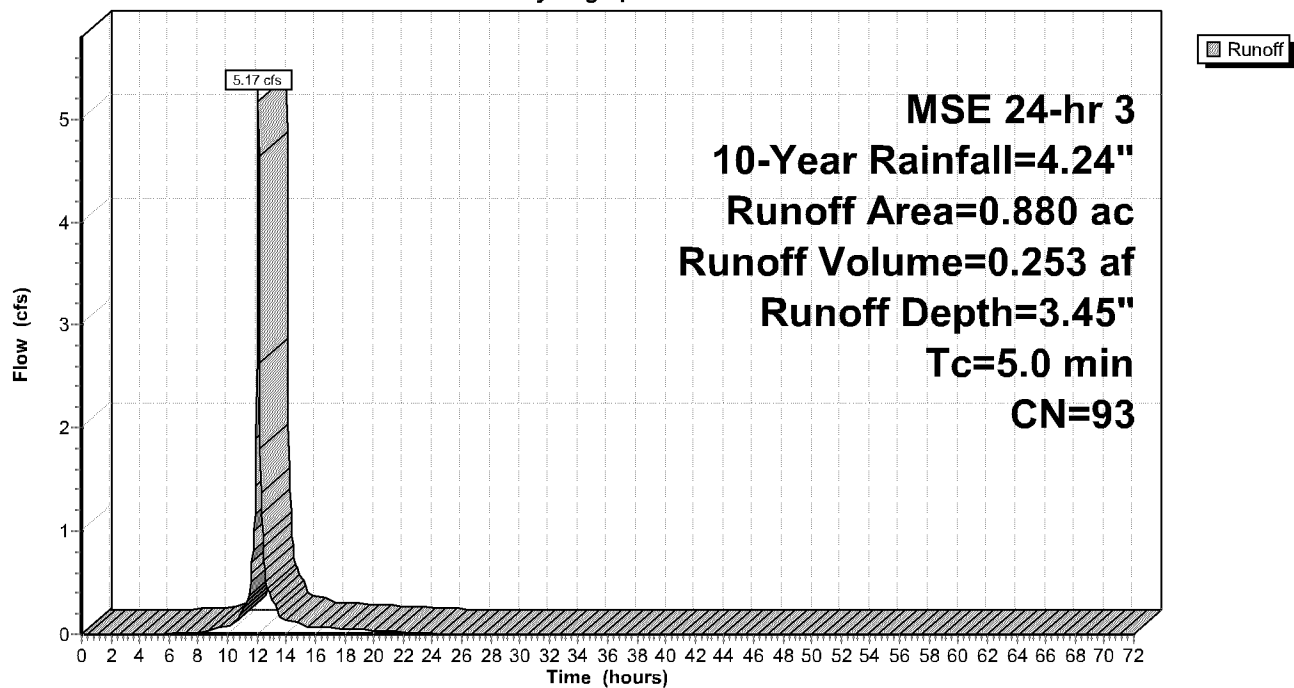
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, $dt=0.05$ hrs
MSE 24-hr 3 10-Year Rainfall=4.24"

Area (ac)	CN	Description
0.810	98	Paved parking, HSG A
0.070	39	>75% Grass cover, Good, HSG A
0.880	93	Weighted Average
0.070		7.95% Pervious Area
0.810		92.05% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment Ex: Existing Area to be Distrurbed

Hydrograph



21618 - HydroCAD - SC740

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MSE 24-hr 3 10-Year Rainfall=4.24"

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Summary for Subcatchment Pr: Proposed Area to be Redeveloped[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 4.26 cfs @ 12.12 hrs, Volume= 0.196 af, Depth= 2.67"

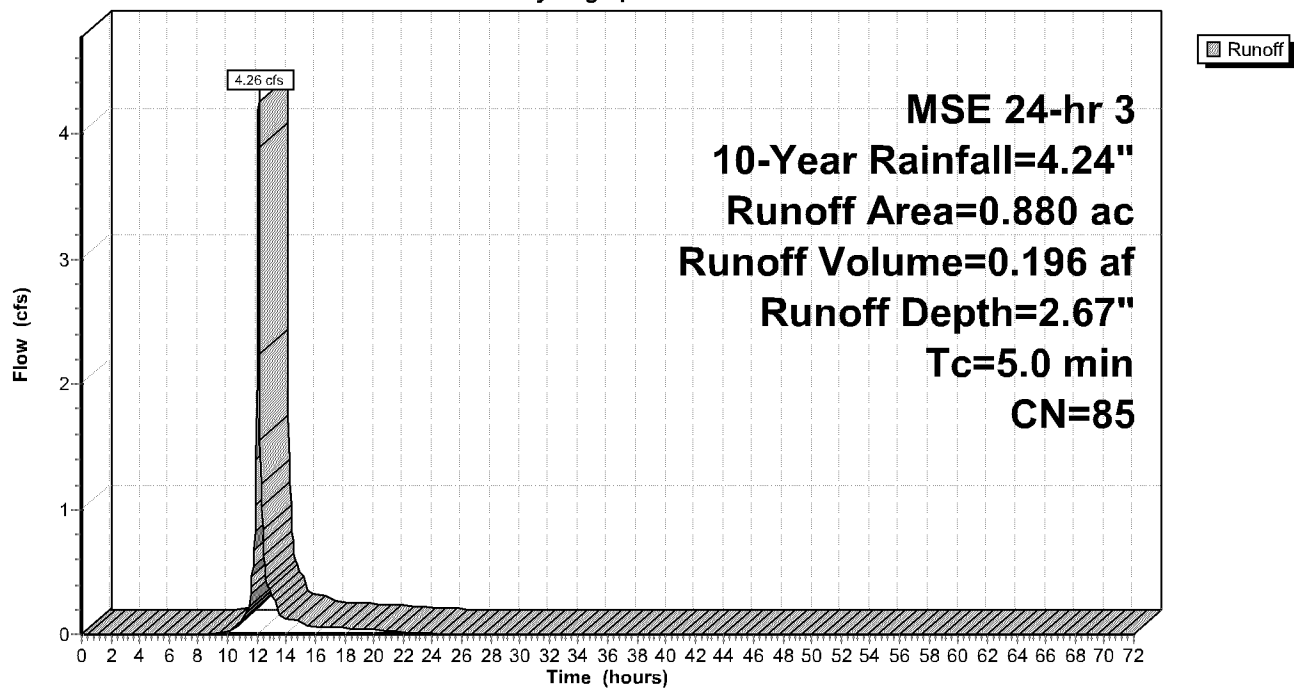
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, $dt=0.05$ hrs
MSE 24-hr 3 10-Year Rainfall=4.24"

Area (ac)	CN	Description
0.690	98	Paved parking, HSG C
0.190	39	>75% Grass cover, Good, HSG A
0.880	85	Weighted Average
0.190		21.59% Pervious Area
0.690		78.41% Impervious Area

T_c (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment Pr: Proposed Area to be Redeveloped

Hydrograph



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MSE 24-hr 3 100-Year Rainfall=7.50"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Ex: Existing Area to be Runoff Area=0.880 ac 92.05% Impervious Runoff Depth=6.67"
Tc=5.0 min CN=93 Runoff=9.58 cfs 0.489 af

Subcatchment Pr: Proposed Area to be Runoff Area=0.880 ac 78.41% Impervious Runoff Depth=5.73"
Tc=5.0 min CN=85 Runoff=8.78 cfs 0.420 af

Total Runoff Area = 1.760 ac Runoff Volume = 0.909 af Average Runoff Depth = 6.20"
14.77% Pervious = 0.260 ac 85.23% Impervious = 1.500 ac

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MSE 24-hr 3 100-Year Rainfall=7.50"

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Summary for Subcatchment Ex: Existing Area to be Distrurbed[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 9.58 cfs @ 12.11 hrs, Volume= 0.489 af, Depth= 6.67"

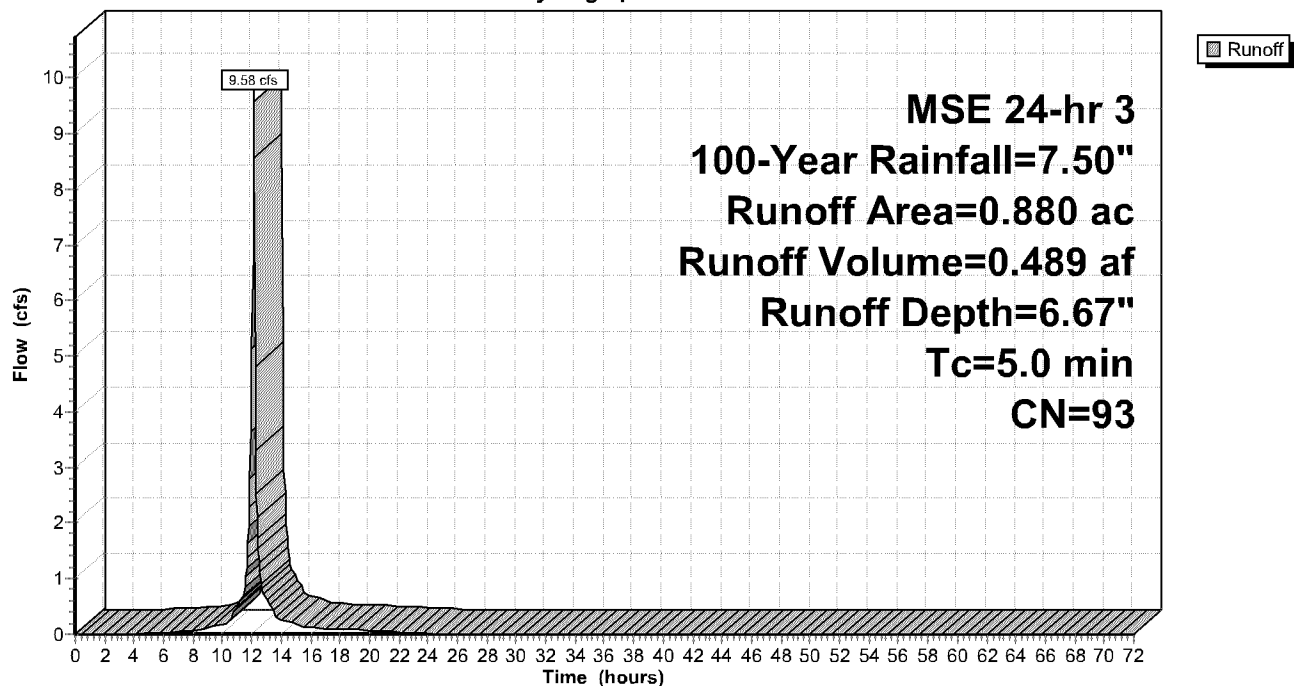
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, $dt=0.05$ hrs
MSE 24-hr 3 100-Year Rainfall=7.50"

Area (ac)	CN	Description
0.810	98	Paved parking, HSG A
0.070	39	>75% Grass cover, Good, HSG A
0.880	93	Weighted Average
0.070		7.95% Pervious Area
0.810		92.05% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment Ex: Existing Area to be Distrurbed

Hydrograph



21618 - HydroCAD - SC740

MSE 24-hr 3 100-Year Rainfall=7.50"

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Summary for Subcatchment Pr: Proposed Area to be Redeveloped[49] Hint: $T_c < 2dt$ may require smaller dt

Runoff = 8.78 cfs @ 12.11 hrs, Volume= 0.420 af, Depth= 5.73"

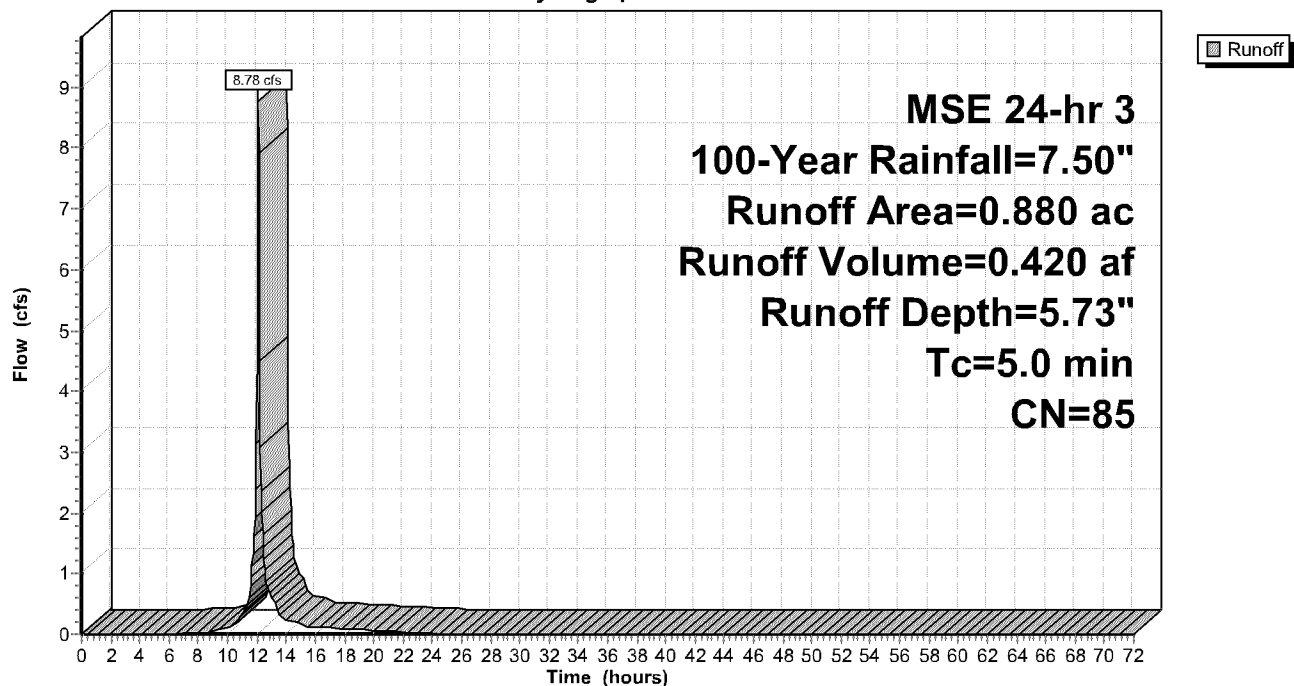
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, $dt=0.05$ hrs
MSE 24-hr 3 100-Year Rainfall=7.50"

Area (ac)	CN	Description
0.690	98	Paved parking, HSG C
0.190	39	>75% Grass cover, Good, HSG A
0.880	85	Weighted Average
0.190		21.59% Pervious Area
0.690		78.41% Impervious Area

T_c (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment Pr: Proposed Area to be Redeveloped

Hydrograph



APPENDIX C – MIDS CALCULATOR RESULTS

Project Information

Calculator Version:	Version 3: January 2017
Project Name:	Valley West End Cap Redevelopment
User Name / Company Name:	Sambatek, Inc.
Date:	December 16, 2020
Project Description:	
Construction Permit?:	No

Site Information

Retention Requirement (inches):	1.1
Site's Zip Code:	55431
Annual Rainfall (inches):	31.3
Phosphorus EMC (mg/l):	0.3
TSS EMC (mg/l):	54.5

Total Site Area

Land Cover	A Soils (acres)	B Soils (acres)	C Soils (acres)	D Soils (acres)	Total (acres)
Forest/Open Space - Undisturbed, protected forest/open space or reforested land	0	0	0	0	0
Managed Turf - disturbed, graded for yards or other turf to be mowed/managed	0.19	0	0	0	0.19
			Impervious Area (acres)		0.69
			Total Area (acres)		0.88

Site Areas Routed to BMPs*

Land Cover	A Soils (acres)	B Soils (acres)	C Soils (acres)	D Soils (acres)	Total (acres)
Forest/Open Space - Undisturbed, protected forest/open space or reforested land					0
Managed Turf - disturbed, graded for yards or other turf to be mowed/managed	0.19				0.19
			Impervious Area (acres)		0.69
			Total Area (acres)		0.88

*NOTE: ACTUAL AREA ROUTED TO BMP IS 0.1 ACRES MANAGED TURF AND 0.85 ACRES IMPERVIOUS. HOWEVER, THE REQUIRED IMPERVIOUS AREA TO BE INFILTRATED IS 0.69 ACRES, WHICH IS WHAT THE BMP IS DESIGNED TO INFILTRATE. MIDS CALCULATOR WILL NOT ALLOW MORE IMPERVIOUS TO BE ROUTED TO A BMP THAN IS INCLUDED IN THE TOTAL SITE AREA.

Summary Information

Performance Goal Requirement

Performance goal volume retention requirement:	2755	ft ³
Volume removed by BMPs towards performance goal:	2755	ft ³
Percent volume removed towards performance goal	100	%

Annual Volume and Pollutant Load Reductions

Post development annual runoff volume	1.6057	acre-ft
Annual runoff volume removed by BMPs:	1.5498	acre-ft
Percent annual runoff volume removed:	97	%

Post development annual particulate P load:	0.721	lbs
Annual particulate P removed by BMPs:	0.696	lbs
Post development annual dissolved P load:	0.59	lbs
Annual dissolved P removed by BMPs:	0.569	lbs
Percent annual total phosphorus removed:	97	%

Post development annual TSS load:	238	lbs
Annual TSS removed by BMPs:	229.7	lbs
Percent annual TSS removed:	97	%

BMP Summary

Performance Goal Summary

BMP Name	BMP Volume Capacity (ft ³)	Volume Received (ft ³)	Volume Retained (ft ³)	Volume Outflow (ft ³)	Percent Retained (%)
1 - Underground infiltration	3474	2755	2755	0	100

Annual Volume Summary

BMP Name	Volume From Direct Watershed (acre-ft)	Volume From Upstream BMPs (acre-ft)	Volume Retained (acre-ft)	Volume outflow (acre-ft)	Percent Retained (%)
1 - Underground infiltration	1.6057	0	1.5498	0.0558999999	97

Particulate Phosphorus Summary

BMP Name	Load From Direct Watershed (lbs)	Load From Upstream BMPs (lbs)	Load Retained (lbs)	Outflow Load (lbs)	Percent Retained (%)
1 - Underground infiltration	0.7206	0	0.6955	0.0251	97

Dissolved Phosphorus Summary

BMP Name	Load From Direct Watershed (lbs)	Load From Upstream BMPs (lbs)	Load Retained (lbs)	Outflow Load (lbs)	Percent Retained (%)
1 - Underground infiltration	0.5896	0	0.5691	0.0205	97

TSS Summary

BMP Name	Load From Direct Watershed (lbs)	Load From Upstream BMPs (lbs)	Load Retained (lbs)	Outflow Load (lbs)	Percent Retained (%)
1 - Underground infiltration	238.03	0	229.74	8.2899999999	97

BMP Schematic

APPENDIX D – DRAFT BORING LOGS



**DENOTES APPROXIMATE LOCATION OF
STANDARD PENETRATION TEST BORING**



20' 0 40'

SCALE: 1"= 40'

**BRAUN
INTERTEC**
The Science You Build On.

11001 Hampshire Avenue S
Minneapolis, MN 55438
952.995.2000
braunintertec.com

Project No:
B1904635

Drawing No:
B1904635

Drawn By: JAG
Date Drawn: 5/8/19
Checked By: RJF
Last Modified: 5/9/19

DRAFT
Subject to change

10606 Vance Avenue S.
Bloomington, Minnesota

**Soil Boring
Location Sketch**

See Descriptive Terminology sheet for explanation of abbreviations

Project Number B1904635 Geotechnical Evaluation Valley West Stormwater System 10606 France Avenue South Bloomington, Minnesota					BORING: ST-1		
					LOCATION: See attached sketch		
					NORTHING:		EASTING:
DRILLER: M. Barber		LOGGED BY: R. Fritz		START DATE: 05/15/19	END DATE: 05/15/19		
SURFACE ELEVATION:		RIG: GP-1	METHOD: 3 1/4" HSA	SURFACING: Bituminous	WEATHER: Sunny		

Elev./ Depth ft	Water Level	Description of Materials (Soil-ASTM D2488 or 2487; Rock-USACE EM 1110-1-2908)	Sample	Blows (N-Value) Recovery	q _p tsf	MC %	Tests or Remarks
0.8		PAVEMENT, 5 inches bituminous over 4 inches aggregate base.					
		POORLY GRADED SAND (SP), fine to medium sand, brown, moist, loose to medium dense (TERRACE DEPOSIT)					
			5	5-5-4 (9) 168"			
				2-2-3 (5) 144"			
			10	4-3-4 (7) 144"			
				5-5-6 (11) 168"			
14.5				7-7-8-10/0" (REF) 204"			
		END OF BORING	15				
		Boring immediately backfilled					
			20				
			25				
			30				

Project Number B1904635 Geotechnical Evaluation Valley West Stormwater System 10606 France Avenue South Bloomington, Minnesota					BORING: ST-2		
					LOCATION: See attached sketch		
					NORTHING:		EASTING:
DRILLER: M. Barber		LOGGED BY: R. Fritz		START DATE: 05/15/19	END DATE: 05/15/19		
SURFACE ELEVATION:		RIG: GP-1	METHOD: 3 1/4" HSA	SURFACING: Bituminous	WEATHER: Sunny		

Elev./ Depth ft	Water Level	Description of Materials (Soil-ASTM D2488 or 2487; Rock-USACE EM 1110-1-2908)	Sample	Blows (N-Value) Recovery	q _p tsf	MC %	Tests or Remarks
0.8		PAVEMENT, 5 inches bituminous over 3 1/2 inches aggregate base.					
4.0		FILL: POORLY GRADED SAND with SILT (SP-SM), fine to medium sand, brown, moist					
		POORLY GRADED SAND (SP), fine to medium sand, brown, moist, loose to medium dense (TERRACE DEPOSIT)					
			5	4-4-3 (7) 156"			
				1-1-1 (2) 144"			
			10	3-3-3 (6) 156"			
				5-3-3 (6) 168"			
14.5				7-7-6-5/0" (REF) 216"			
		END OF BORING	15				
		Boring immediately backfilled					
			20				
			25				
			30				

APPENDIX E – MAINTENANCE AGREEMENT



**VALLEY WEST SHOPPING CENTER
END CAP REDEVELOPMENT
MAINTENANCE PLAN**

Project Name & Address: Valley West Shopping Center End Cap Redevelopment

10520 France Avenue South, Bloomington MN 55431

**LIST OF VALLEY WEST SHOPPING CENTER END CAP REDEVELOPMENT STORMWATER
MANAGEMENT PRACTICES**

1. StormTech Underground Infiltration Basin w/ Isolator Row
2. Standard Catch Basin Manhole w/ Sump
3. Storm Sewer Pipe & Standard Manholes

RESPONSIBLE PARTY FOR MAINTENANCE OF STORMWATER MANAGEMENT PRACTICES

The OWNER will be responsible for ongoing maintenance of the stormwater management practices. The OWNER for the site is:

Kraus Anderson
501 S 8th Street
Minneapolis, MN 55404
TEL: 612-336-6410
Contact: Tim Marco

NOW THEREFORE, the OWNER makes this agreement and hereby declares that this agreement shall constitute covenants to run with the Property, and further declares that the Property shall be owned, used, occupied, and conveyed subject to the covenants and restrictions set forth in this declaration, all of which shall be binding in perpetuity on all persons owning or acquiring any right, title or interest in the Property, and their heirs, successors, personal representative and assigns, but only during the period of ownership of that right, title of interest.

1. OWNER will inspect the Facilities at least annually.

2. OWNER will maintain and repair the Facilities:
 - a. In the case of the basins and other facilities where sediment collects, to preserve live storage or capacity at or above the design volume or, where no design live storage volume or capacity is incorporated into the permit, the volume or capacity recommended by the manufacturer.
 - b. In the case of conveyances and other structures, to preserve design hydraulic capacity.
 - c. In the case of all facilities, as necessary to preserve the integrity and intended function of the facility.

FREQUENCY OF INSPECTION, INSPECTION ACTIVITIES, AND MAINTENANCE ACTIVITIES

1. StormTech Underground Infiltration Basin w/ Isolator Row

Frequency of Inspection/Indicator of Maintenance: Semi-Annually/Average depth of sediment exceeds 3" throughout isolator row

Description of Inspection Activities: See Appendix B for manufacturer's guidelines

Description of Maintenance Activities: See Appendix B for manufacturer's guidelines

2. Standard Catch Basin Manhole w/ Sump

Frequency of Inspection/Indicator of Maintenance: Semi-Annually/Average depth of sediment exceeds 6" in the sump

Description of Inspection Activities: Practice should be inspected (and cleaned out if deemed necessary) semi-annually to ensure that sediment is not beginning to wash out during storm events. Pipes and grate should be inspected to ensure no debris is causing blockage.

Description of Maintenance Activities: Practice should be cleaned out and sediment should be removed once per year. Remove any blockages or obstructions in pipes or grate.

5. Storm Sewer Pipe & Standard Manholes

Frequency of Inspection/Indicator of Maintenance: Annually/manholes and catch basins are flooding.

Description of Inspection Activities: Practice should be inspected (and cleaned out if deemed necessary) annually to ensure that pipe conveyance is maintained during storm events. Pipes should be inspected to ensure no debris is causing blockage.

Description of Maintenance Activities: Practice should be cleaned out and sediment should be removed once per year. Remove any blockages or obstructions in pipes.

Drafted by:
Sambatek, Inc.
12800 Whitewater Drive, Suite 300
Minnetonka, MN 55343

APPENDIX A – STORMWATER MANAGEMENT PRACTICES EXHIBIT



12800 Whitewater Drive, Suite 300
Minnetonka, MN 55343
763.476.6010 telephone
763.476.8532 facsimile

Engineering | Surveying | Planning | Environmental

Client
**KRAUS
ANDERSON**

501 S 8TH STREET
MINNEAPOLIS, MN 55404

Project
**VALLEY WEST
SHOPPING
CENTER END
CAP**

Location
**BLOOMINGTON,
MN**
10520 FRANCE AVENUE S.

Certification

I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly licensed professional ENGINEER under the laws of the state of Minnesota.

Brady D. Busselman
Brady D. Busselman

Registration No. 44579 Date: 10/19/2020

If applicable, contact us for a wet signed copy of this plan which is available upon request at Sambatek's, Minnetonka, MN office.

Summary

Designed: AR Drawn: JMW
Approved: BOB Book / Page:
Phase: PRELIMINARY Initial Issue: 10/19/2020

Revision History

No.	Date	By	Submittal / Revision
09/30/20	JGP	CUP/Permit Set	
10/19/20	JMW	Permit Plans	
12/11/20	JGP	City Comment Response	

Sheet Title
**GRADING,
DRAINAGE &
UTILITY PLAN**

Sheet No. Revision
C4.01

Project No. 21618

LEGEND

PROPERTY LIMIT

CURB & GUTTER

STORM SEWER

DRAIN TILE

BUILDING

RETAINING WALL

WETLAND LIMITS

TREELINE

SPOT ELEVATION

CONTOUR

RIP RAP

OVERFLOW ELEV.

SOIL BORINGS

SPOT ELEVATION-MATCH

EXISTING (FIELD VERIFY

AND CONTACT ENGINEER

IF DISCREPANCIES EXIST)

PROPOSED

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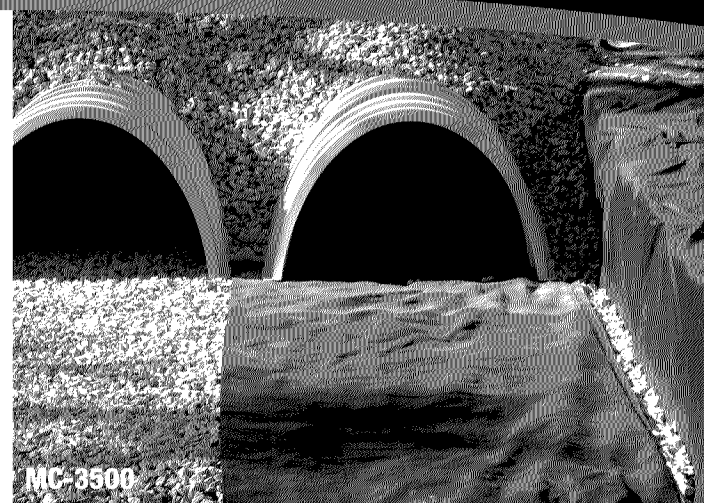
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APPENDIX B – STORMTECH INSPECTION AND MAINTENANCE

Isolator® Row O&M Manual



THE ISOLATOR[®] ROW

INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a technique to inexpensively enhance Total Suspended Solids (TSS) and Total Phosphorus (TP) removal with easy access for inspection and maintenance.

THE ISOLATOR ROW

The Isolator Row is a row of StormTech chambers, either SC-160, SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-4500 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC-310-3 and SC-740 models) allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.

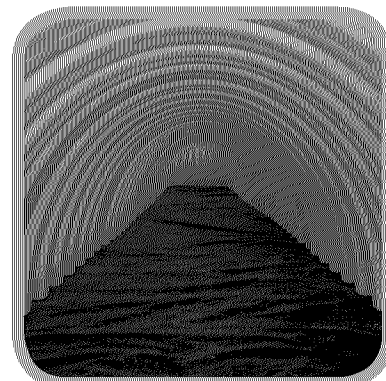
A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The woven geotextile provides a media for stormwater filtration, a durable surface for maintenance, prevents scour of the underlying stone and remains intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber. The non-woven fabric is not required over the SC-160, DC-780, MC-3500 or MC-4500 models as these chambers do not have perforated side walls.

The Isolator Row is typically designed to capture the “first flush” and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole provides access to the Isolator Row and typically includes a high flow weir. When flow rates or volumes exceed the Isolator Row weir capacity the water will flow over the weir and discharge through a manifold to the other chambers.

Another acceptable design uses one open grate inlet structure. Using a “high/low” design (low invert elevation on the Isolator Row and a higher invert elevation on the manifold) an open grate structure can provide the advantages of the Isolator Row by creating a differential between the Isolator Row and manifold thus allowing for settlement in the Isolator Row.

The Isolator Row may be part of a treatment train system. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.

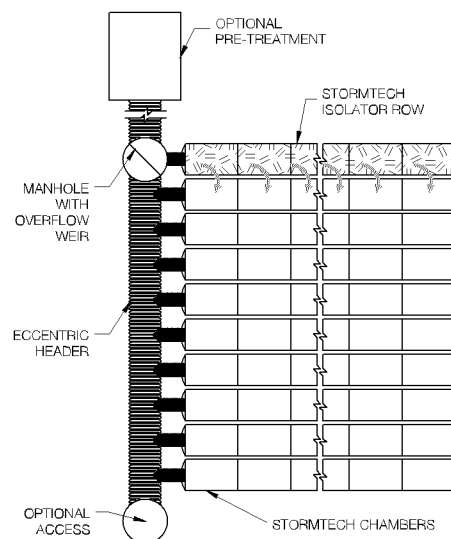
Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.

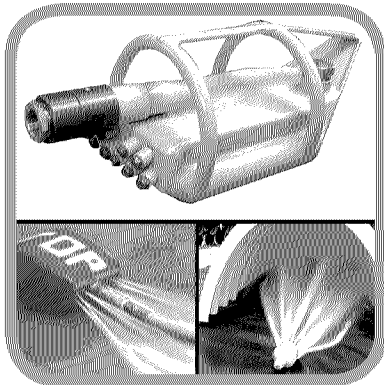


Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.



StormTech Isolator Row with Overflow Spillway (not to scale)





ISOLATOR ROW INSPECTION/MAINTENANCE

INSPECTION

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

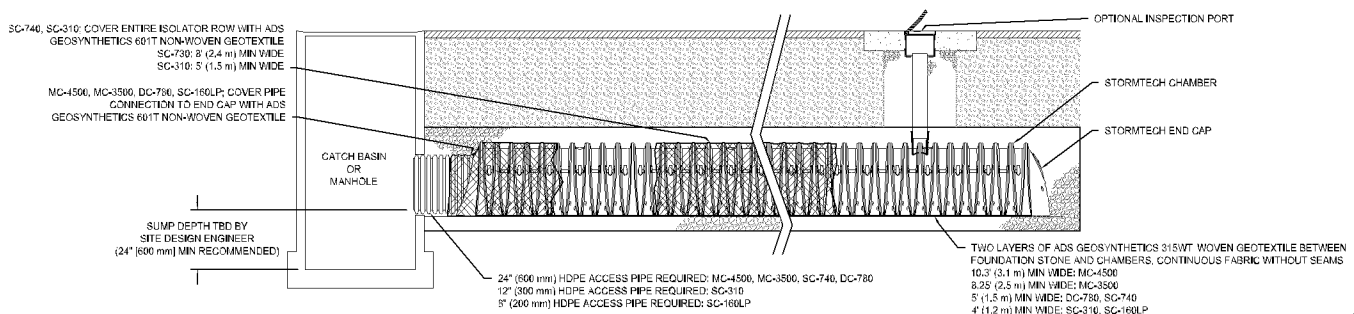
MAINTENANCE

The Isolator Row was designed to reduce the cost of periodic maintenance. By “isolating” sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45° are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. **The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.**

StormTech Isolator Row (not to scale)

Note: Non-woven fabric is only required over the inlet pipe connection into the end cap for SC-160LP, DC-780, MC-3500 and MC-4500 chamber models and is not required over the entire Isolator Row.



ISOLATOR ROW STEP BY STEP MAINTENANCE PROCEDURES

STEP 1

Inspect Isolator Row for sediment.

A) Inspection ports (if present)

- i. Remove lid from floor box frame
- ii. Remove cap from inspection riser
- iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.
- iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.

B) All Isolator Rows

- i. Remove cover from manhole at upstream end of Isolator Row
- ii. Using a flashlight, inspect down Isolator Row through outlet pipe
 1. Mirrors on poles or cameras may be used to avoid a confined space entry
 2. Follow OSHA regulations for confined space entry if entering manhole
- iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2. If not, proceed to Step 3.

STEP 2

Clean out Isolator Row using the JetVac process.

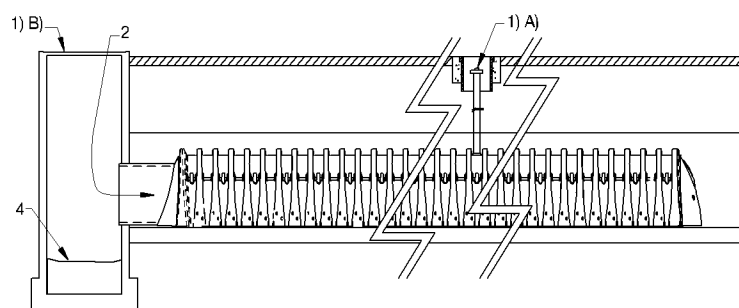
- A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

STEP 3

Replace all caps, lids and covers, record observations and actions.

STEP 4

Inspect & clean catch basins and manholes upstream of the StormTech system.



SAMPLE MAINTENANCE LOG

Date	Stadia Rod Readings		Sediment Depth (1)-(2)	Observations/Actions	Inspector
	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)			
3/15/11	6.3 ft	none		New installation. Fixed point is CI frame at grade	DJM
9/24/11		6.2	0.1 ft	Some grit felt	SM
6/20/13		5.8	0.5 ft	Mucky feel, debris visible in manhole and in Isolator Row, maintenance due	NV
7/7/13	6.3 ft		0	System jetted and vacuumed	DJM